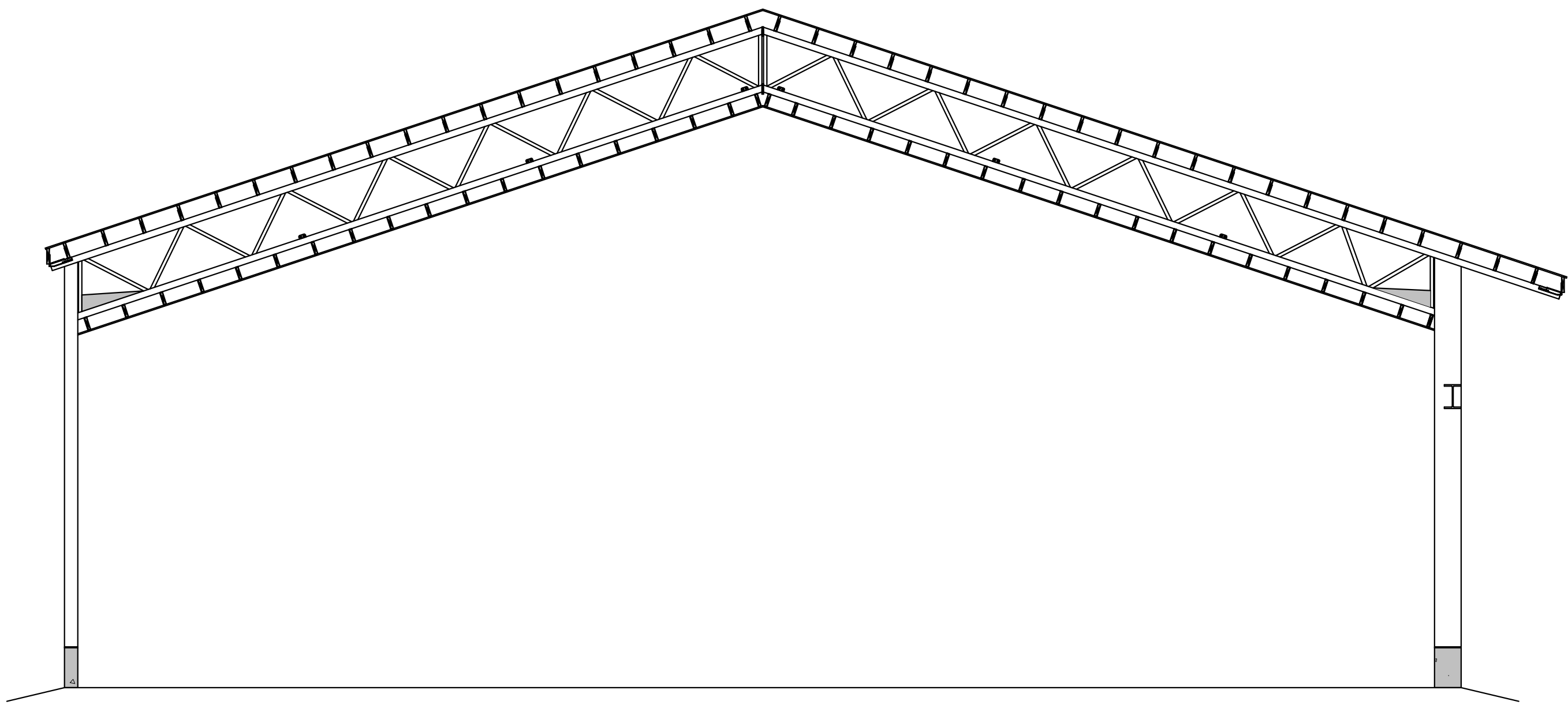


70' x 120' RIDGELINE BUILDING FOR:
CORNERSTONE PRESSURE WASHING
5382 LAYTON ST. WYOMING, ON N0N 1T0



BUILDING MANUFACTURER:
WECOVER STRUCTURES INC.
22694 VICTORIA ROAD,
THAMESVILLE, ON N0P 2K0

DRAWING INDEX:

- S1 - TITLE PAGE
- S2 - ROOF BRACING PLAN
- S3 - TRUSS CROSS SECTION
- S4 - TRUSS CROSS SECTION
- S5 - PLATES AND CONNECTIONS
- S6 - PLATES AND CONNECTIONS
- S7 - PLATES AND CONNECTIONS
- S8 - END WALL ELEVATIONS
- S9 - END WALL ELEVATIONS
- S10 - PLATES AND CONNECTIONS
- S11 - SIDE WALL ELEVATIONS
- S12 - HEADER DETAILS
- S13 - GENERAL NOTES
- F1 - FOUNDATION PLAN
- F2 - FOUNDATION DETAILS
- F3 - FOUNDATION DETAILS

THIS PROJECT HAS BEEN DESIGNED AND FABRICATED IN ACCORDANCE WITH THE FOLLOWING INFORMATION:

CUSTOMER NAME & SITE LOCATION:

CORNERSTONE PRESSURE WASHING
5382 LAYTON ST. WYOMING, ON N0N 1T0

BUILDING TYPE:

RIDGELINE BUILDING

BUILDING SIZE:

70' x 120' Bay Spacing Varies

APPLICABLE BUILDING CODE:

Designed In accordance with

INTENDED USE:

NBC 2015 & National Farm Building Code of Canada and OBC 2012

OCCUPANCY:

STORAGE

LOW OCCUPANCY COMMERCIAL (F2-LOW HAZARD)

THE INTENT OF THIS DRAWING SET IS TO CONVEY THE BUILDING GEOMETRY, DESIGN CRITERIA, STRUCTURAL DETAILS AND SPECIFICATIONS. FINAL DETAILING OF PLATES AND MEMBERS FOR THIS BUILDING IS TO BE COMPLETED ON THE SHOP DRAWINGS.

STRUCTURAL DESIGN CRITERIA:
THIS PROJECT HAS BEEN DESIGNED AND FABRICATED IN ACCORDANCE WITH THE FOLLOWING:

- A) DEAD LOADS

I) SELF-WEIGHT OF BUILDING COMPONENTS

II) COLLATERAL (HANGING) LOAD, NOT TO EXCEED 0.5 PSF (0.024 kPa) AS AN ALLOWANCE FOR MECHANICAL, ELECTRICAL CEILING, SPRINKLERS, ETC, OR ANY COMBINATION THERE OF
- B) LIVE LOADS

LIVE LOADS DETERMINED IN ACCORDANCE WITH SECTION 4.1.5 OF NBC 2015

MINIMUM ROOF LIVE LOAD IS 20.89 PSF (1.0 kPa)

MEZZANINE LIVE LOAD IS 100 PSF (4.8 kPa)
- C) SNOW LOADS

SNOW LOADS DETERMINED IN ACCORDANCE WITH SECTION 4.1.6 OF NBC & COMMENTARY G OF NBC 2015 STRUCTURAL COMMENTARIES

IMPORTANCE FACTOR-
GROUND SNOW LOAD-
ASSOCIATED RAIN LOAD-
WIND EXPOSURE FACTOR-
ACCUMULATION FACTOR-
ROOF SNOW LOAD -

Is= 0.8
Ss= 1.6 kPa
Sr= 0.4 kPa
Cw= 0.75
Ca= 1.0 (1.15 FOR UNBALANCED)
S= 1.45 kPa, (Case 2 for unbalanced loading)

BUILDING EXPOSURE: OPEN TERRAIN

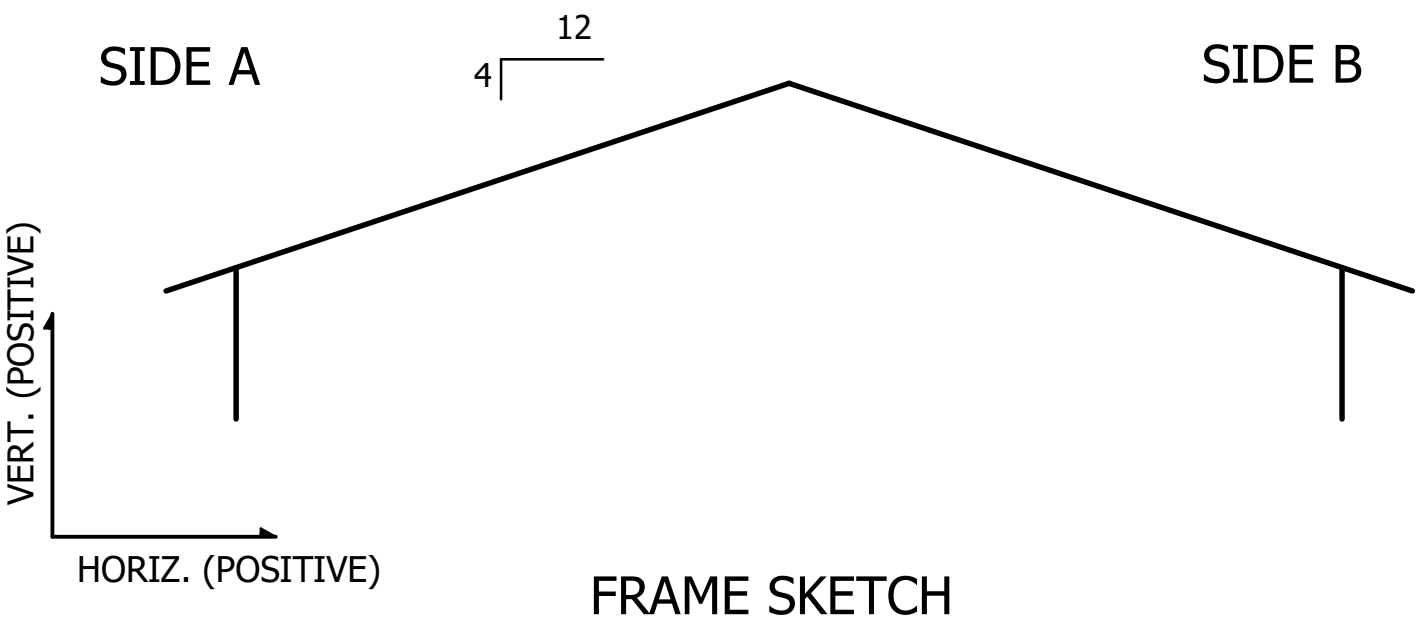
- D) WIND LOADS

WIND LOADS DETERMINED IN ACCORDANCE WITH SECTION 4.1.7 OF NBC & COMMENTARY I OF NBC 2015 STRUCTURAL COMMENTARIES

IMPORTANCE FACTORS
REFERENCE WIND PRESSURE
EXPOSURE FACTOR
BUILDING ENCLOSURE

Iw= 0.8
q(1/10)= 0.47 kPa
Ce = 0.90
FULLY ENCLOSED (CATEGORY 2)

- E) LOAD COMBINATIONS DETERMINED IN ACCORDANCE WITH SECTION 4.1.3 OF NBC & COMMENTARY A OF THE NBC 2015 STRUCTURAL COMMENTARIES



BASE REACTIONS (UNFACTORED) :The maximum forces at the foundation/supports								
SIDE A(Windward)					SIDE B (Leeward - with headers)			
LOAD CASE:	HORIZONTAL		VERTICAL		HORIZONTAL		VERTICAL	
	X _A (kips)	X _A (kN)	Y _A (kips)	Y _A (kN)	X _B (kips)	X _B (kN)	Y _B (kips)	Y _B (kN)
DEAD	1.96	8.71	7.31	32.54	-3.92	-17.42	14.63	65.08
LIVE	3.15	14.00	11.28	50.19	-6.29	-28.00	22.56	100.38
SNOW BALANCED	3.45	15.36	13.07	58.16	-6.91	-30.72	26.15	116.32
SNOW UNBALANCED	2.32	10.32	13.77	61.27	-4.64	-20.64	27.54	122.54
WIND: PERPENDICULAR	-2.75	-12.22	-5.84	-25.96	-0.56	-2.50	-11.67	-51.92
WIND:PARALLEL	0.15	0.67	-5.26	-23.38	1.01	4.50	-10.51	-46.76
Note:								
Negative values of Y reactions indicates uplift forces.								
Parallel Wind (Endwalls): 0.7 Kpa								

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03/16/2022	FLOOR PLAN LAYOUT REVISED

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206 SOUTH MAIN STREET
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DO NOT SCALE DRAWINGS

CLIENT:

CORNERSTONE PRESSURE WASHING

PROJECT NAME:

CORNERSTONE PRESSURE WASHING

PROJECT LOCATION:

5382 LAYTON ST. WYOMING, ON N0N 1T0

PROJECT STATUS & VERSION:

STRUCTURAL DESIGN DRAWINGS

DRAWN BY:
CHRISTIAN

CHECKED BY:
RAMA

PAGE DESCRIPTION:

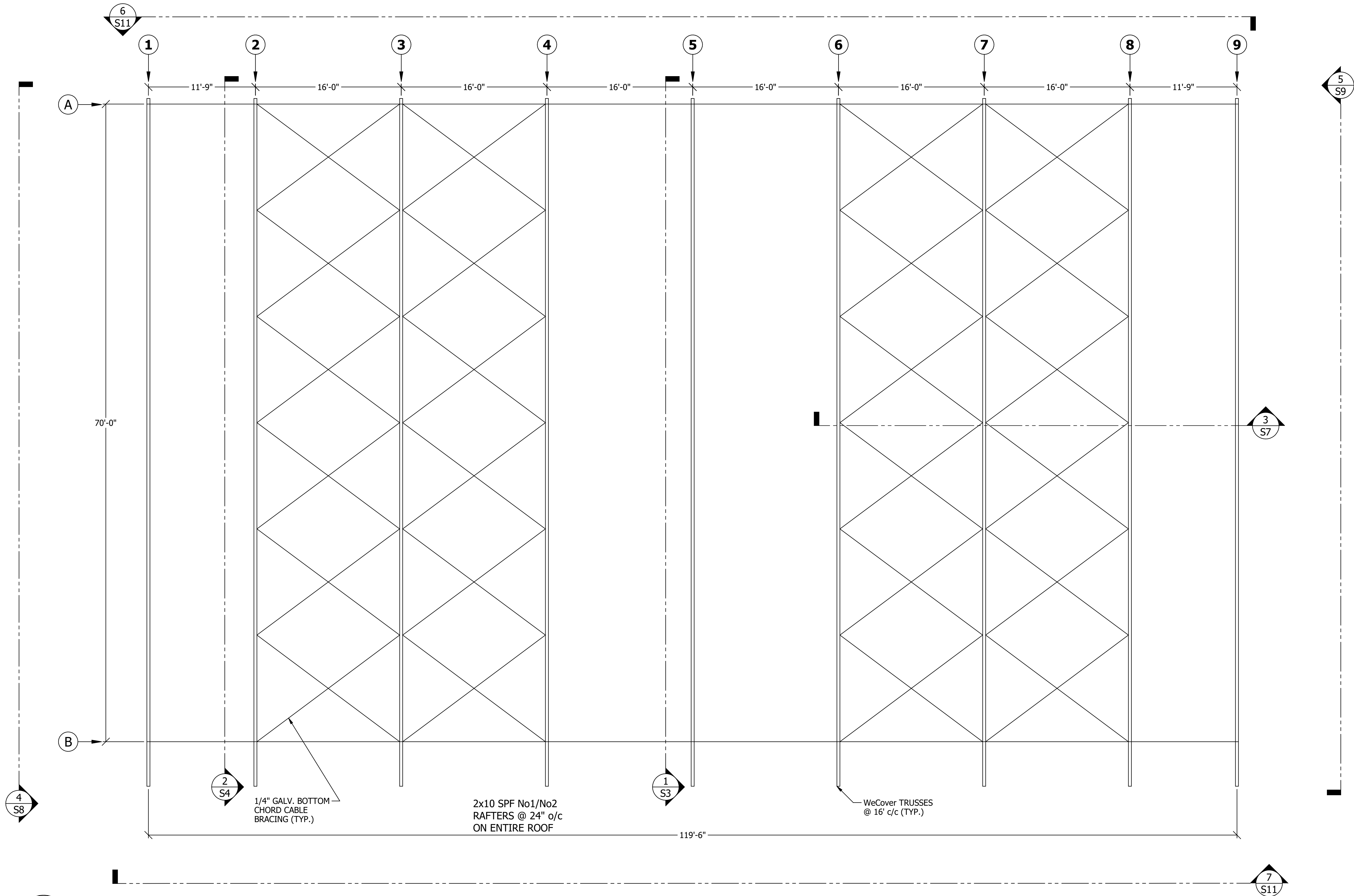
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PROJECT START DATE:
6/23/2021

LAST PRINT DATE:
03/16/2022

PROJECT/ CONTRACT NO.
WECOVER 21-033
RIDGELINE 101.21.2115-ADD2

SHEET NO.



ROOF BRACING PLAN
S2 SCALE: 3/16" = 1'

ROOF FRAMING LINE LEGEND	
	TRUSSES
	PURLINS
	45° ANGLE BRACE

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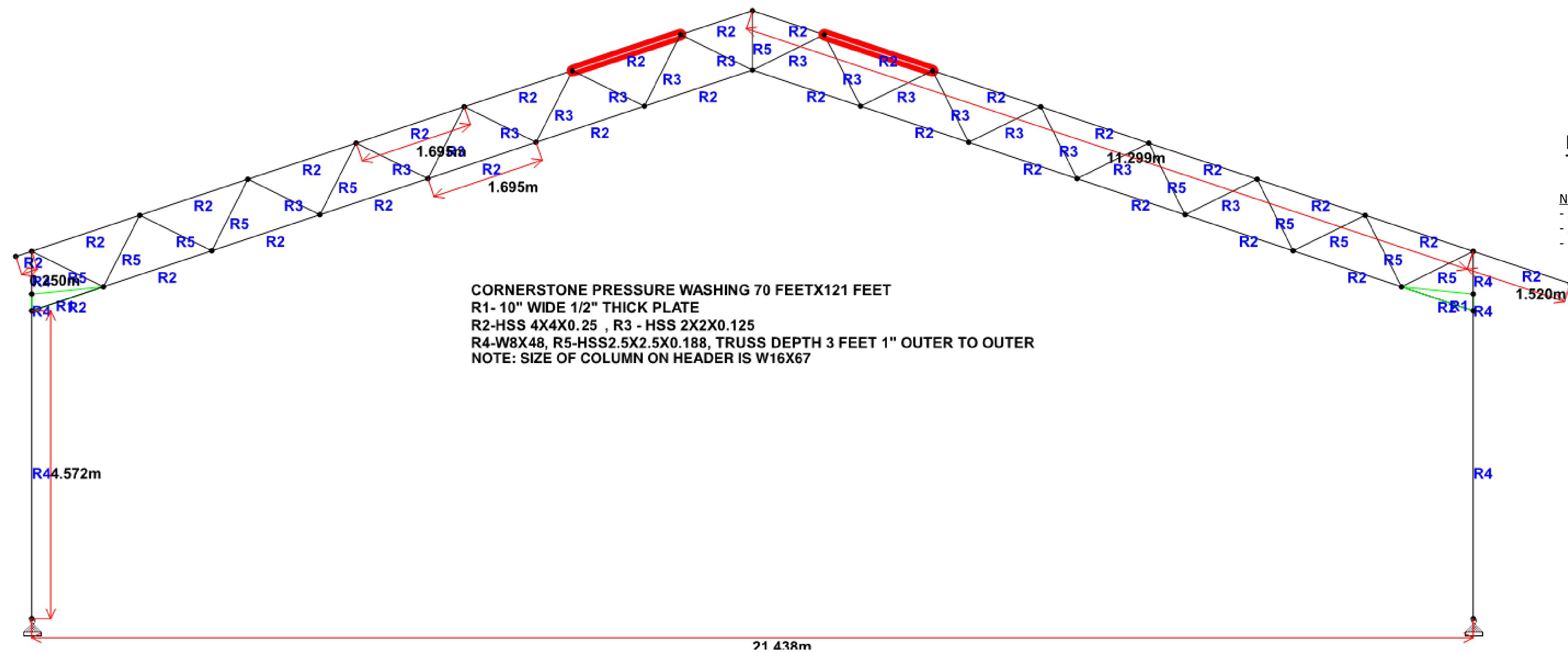
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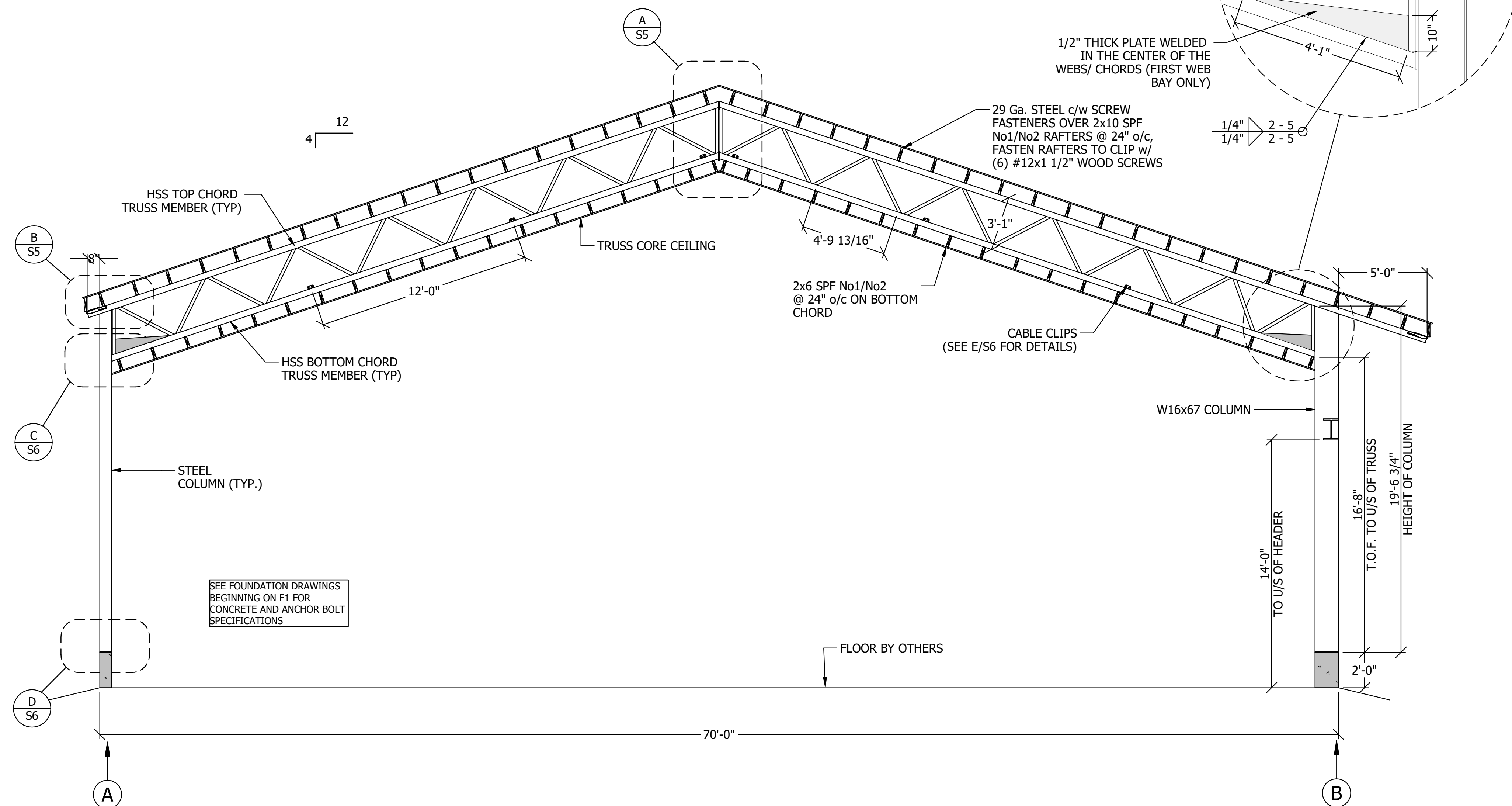
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PROJECT NAME: CORNERSTONE PRESSURE WASHING	
PROJECT LOCATION: 5382 LAYTON ST. WYOMING, ON NON 1T0	
PROJECT STATUS & VERSION: STRUCTURAL DESIGN DRAWINGS	

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PROJECT/ CONTRACT NO. WECOVER 21-033 RIDGELINE 101.21.2115-ADD2	
SHEET NO.	



MEMBER SIZE DIAGRAM

NOTE
- ALL WEB MEMBERS HSS 2"x2"x1/8" U.N.O.
- ALL BOLTS ARE SAE J429- GRD. 8 U.N.O.
- HOLLOW STRUCTURAL SECTIONS (HSS) SIZING KEY: INCH x INCH x SIXTEENTHS



1
S3 **CROSS SECTION (TRUSS 5)**
SCALE: 1/4" = 1'-0"

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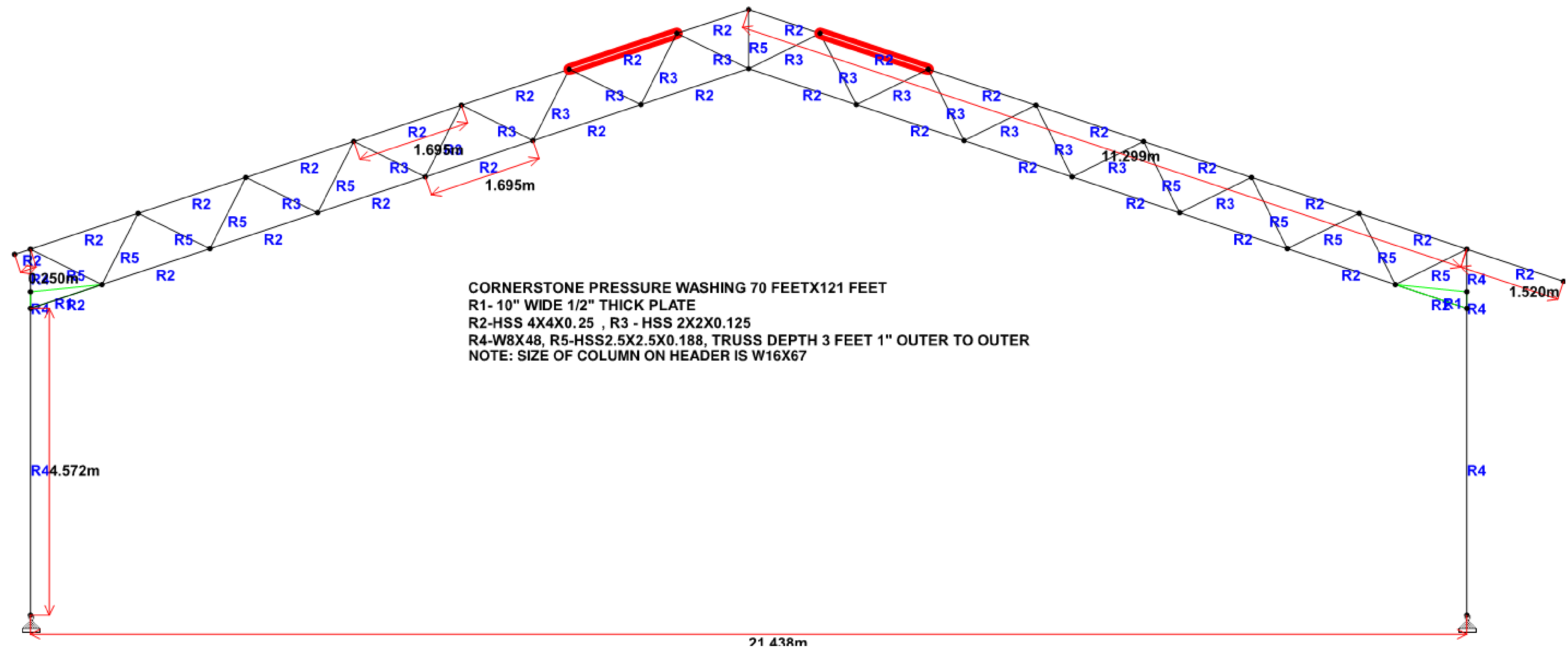
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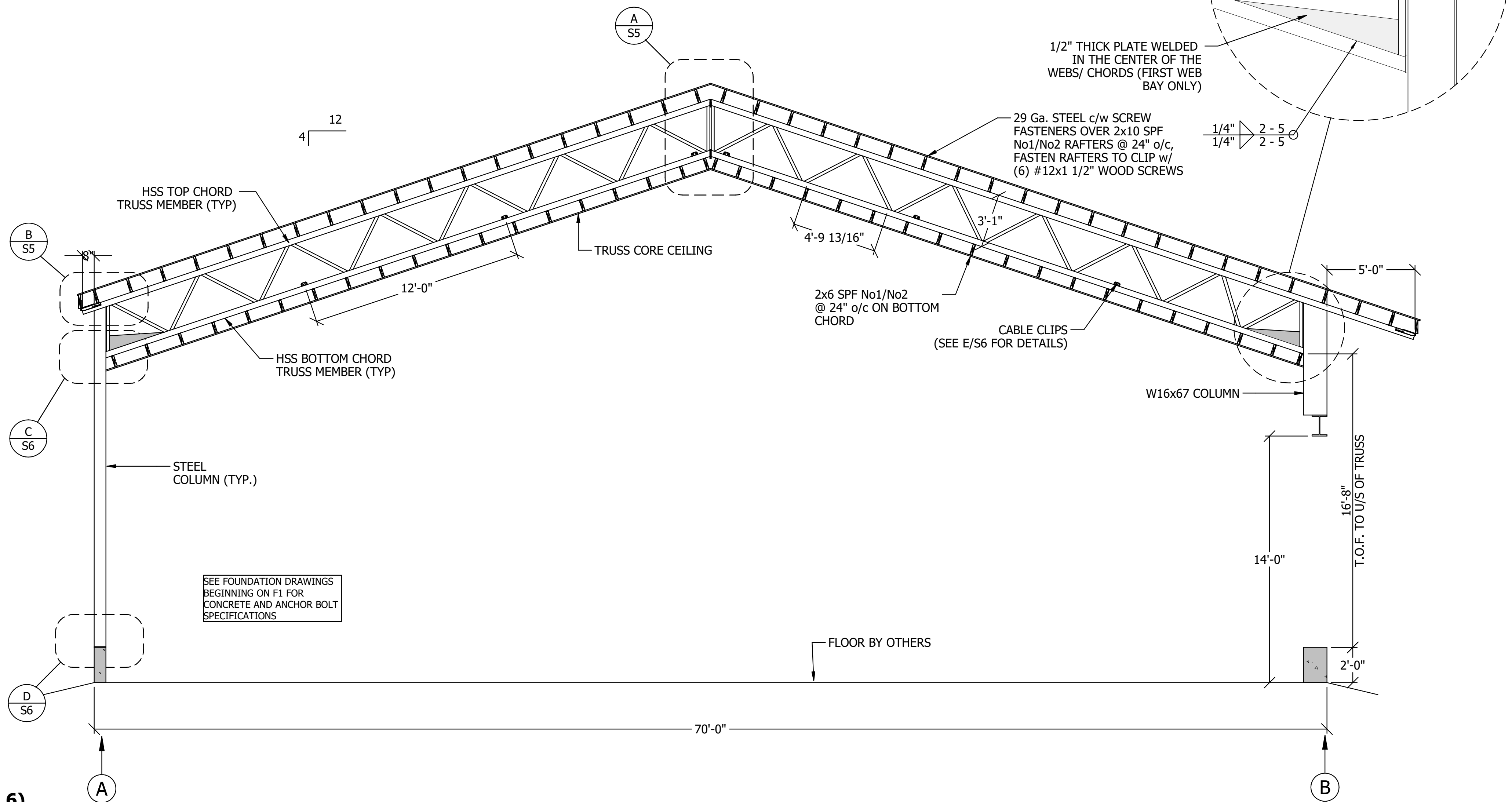
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SHEET NO.	

S3



MEMBER SIZE DIAGRAM

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- HOLLOW STRUCTURAL SECTIONS (HSS) SIZING KEY: INCH x INCH x SIXTEENTHS



2
S4 **CROSS SECTION (TRUSS 2,4,6)**
SCALE: 1/4" = 1'-0"

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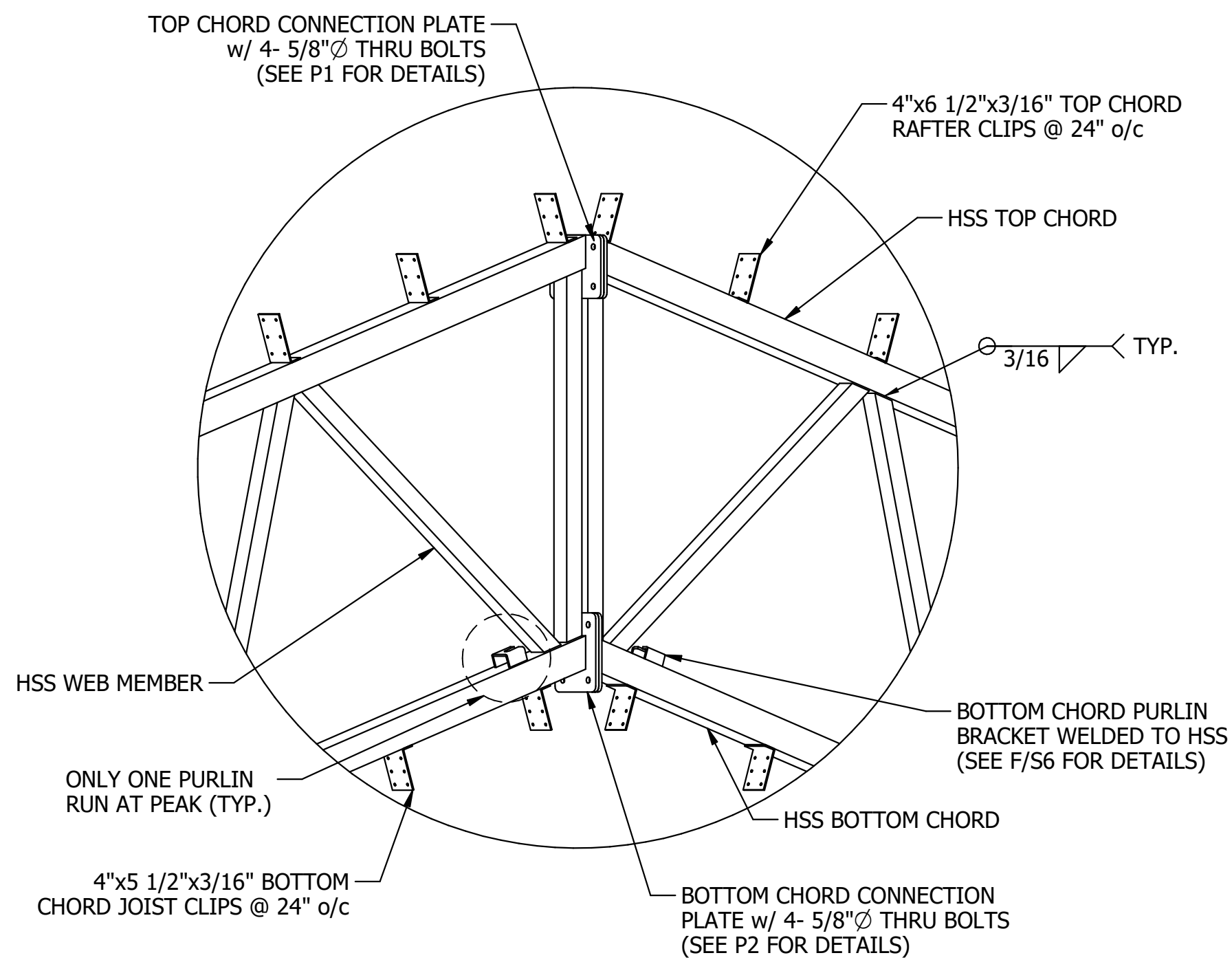
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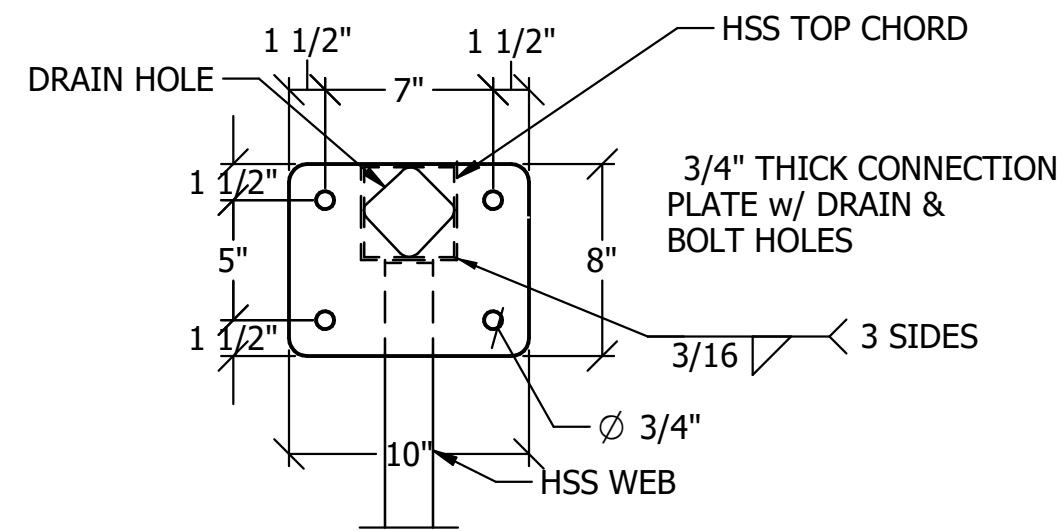
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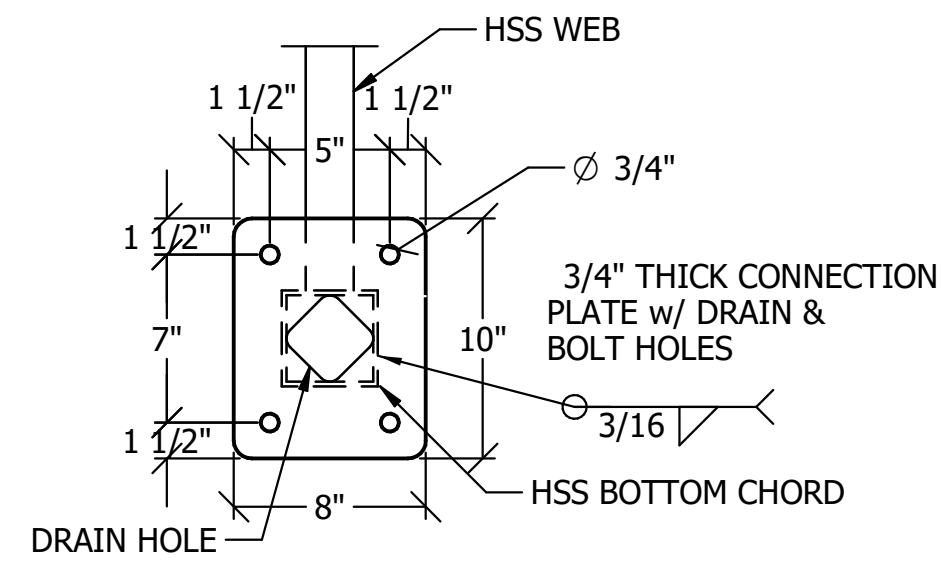
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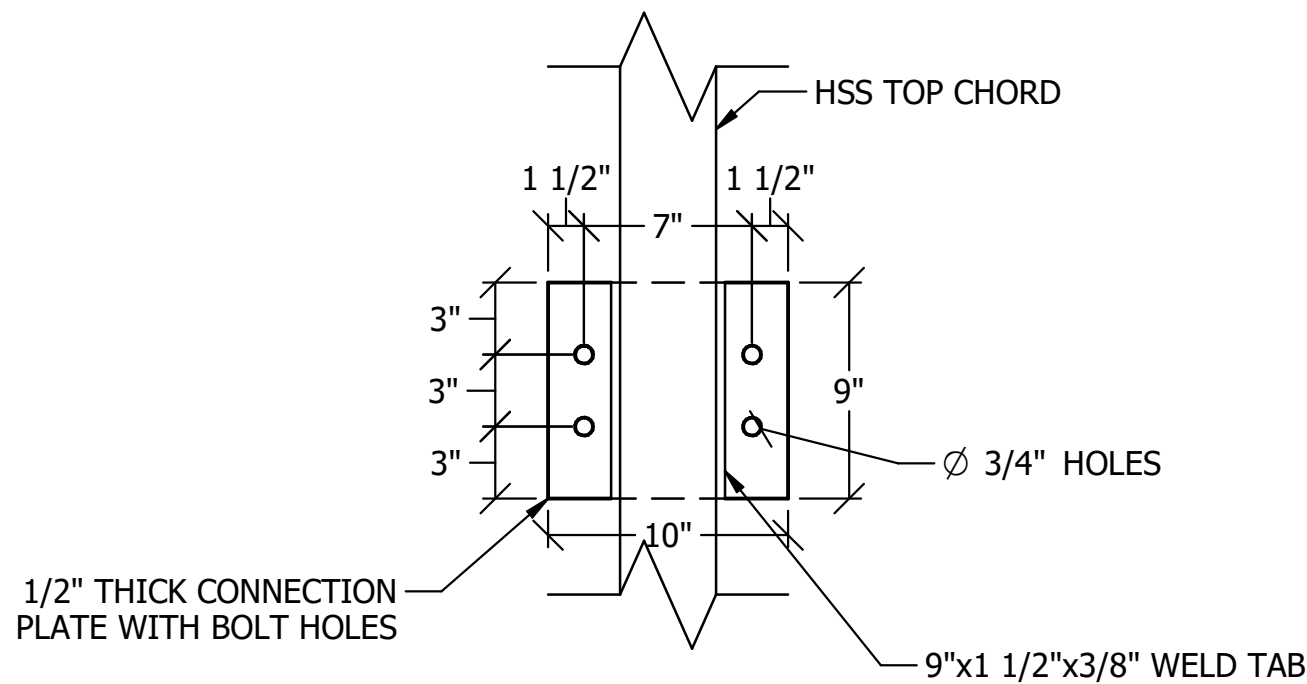
A
S5 **TRUSS PEAK DETAIL**



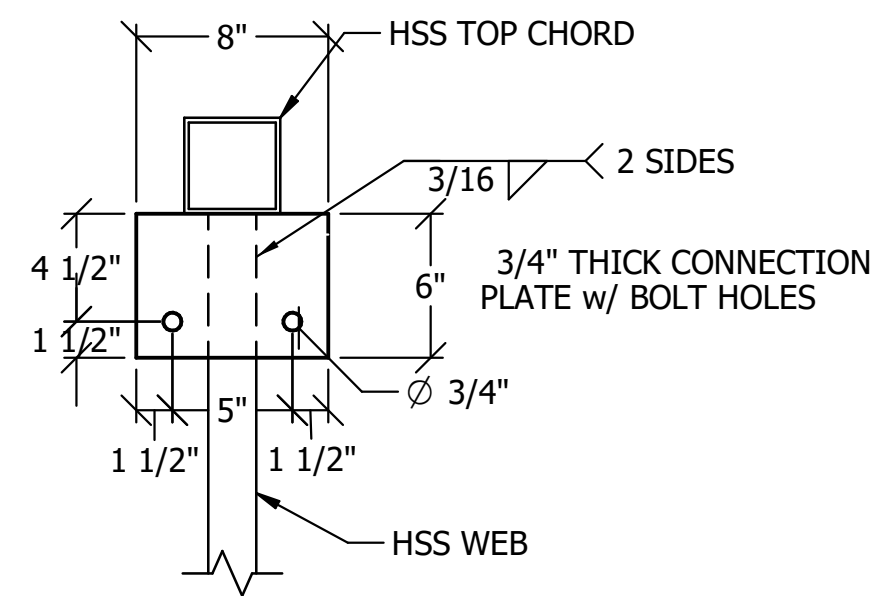
P1 **TOP CHORD CONNECTION PLATE**
@ PEAK - SCALE: 1 1/2"=1'-0"



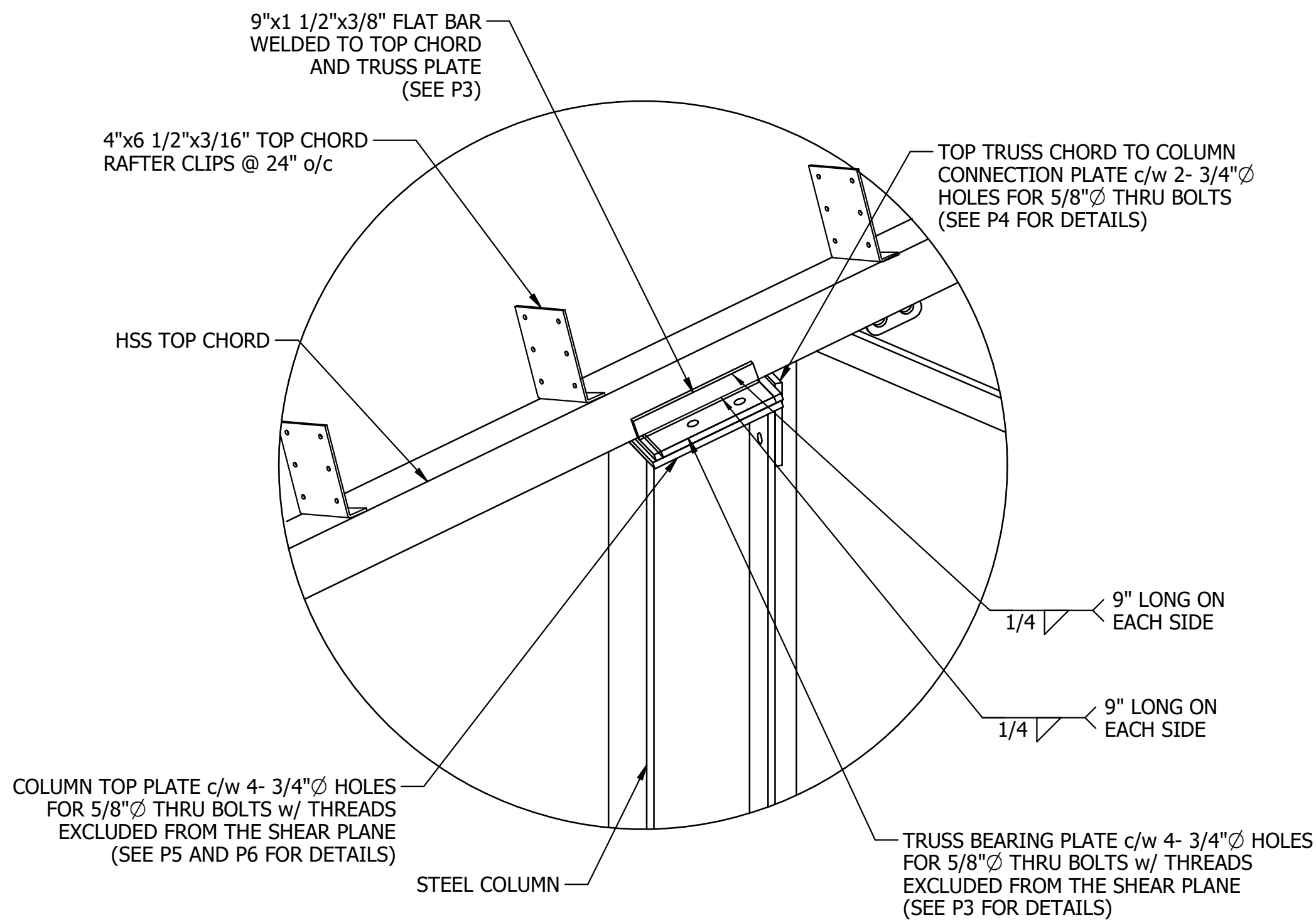
P2 **BOTTOM CHORD CONNECTION PLATE**
@ PEAK - SCALE: 1 1/2"=1'-0"



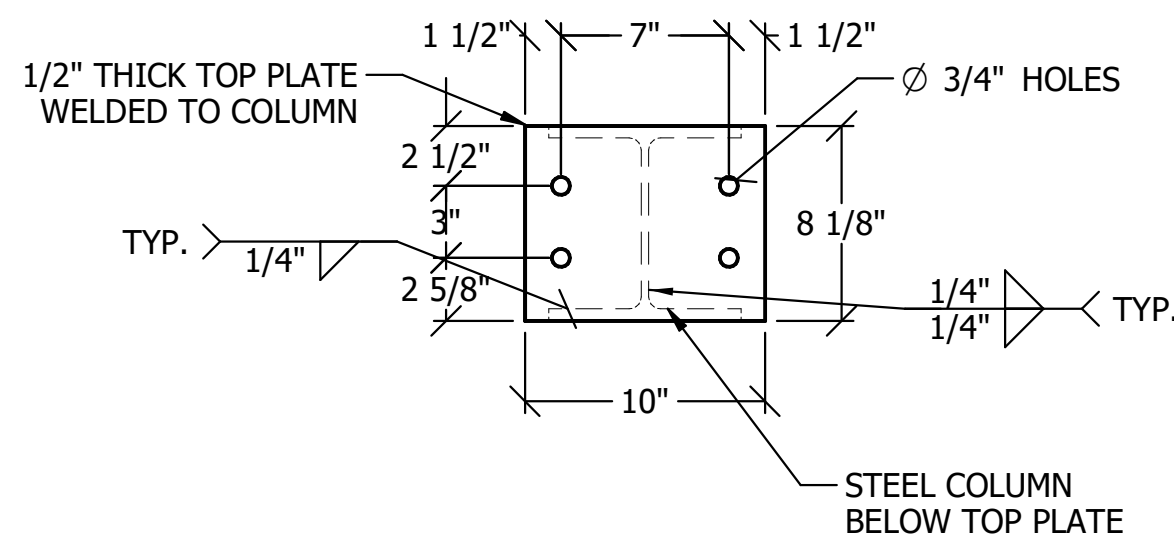
P3 **TOP CHORD TO COLUMN TOP PLATE**
SCALE: 1 1/2"=1'-0"



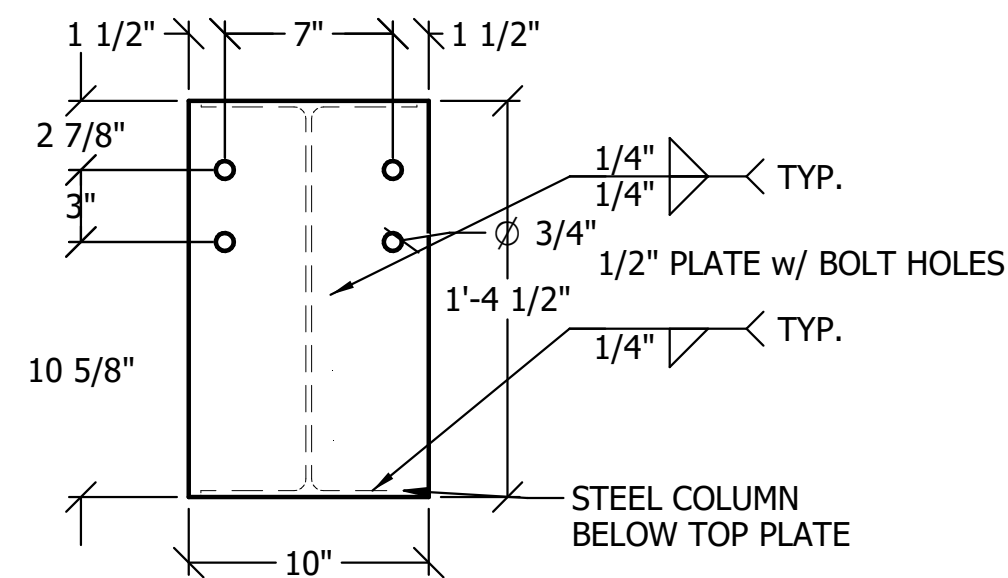
P4 **TOP CHORD TO COLUMN SIDE PLATE**
SCALE: 1 1/2"=1'-0"



B
S5 **TRUSS/COLUMN CONNECTION DETAIL**



P5 **COLUMN TOP PLATE (W8)**
SCALE: 1 1/2"=1'-0"



P6 **COLUMN TOP PLATE (W16)**
SCALE: 1 1/2"=1'-0"

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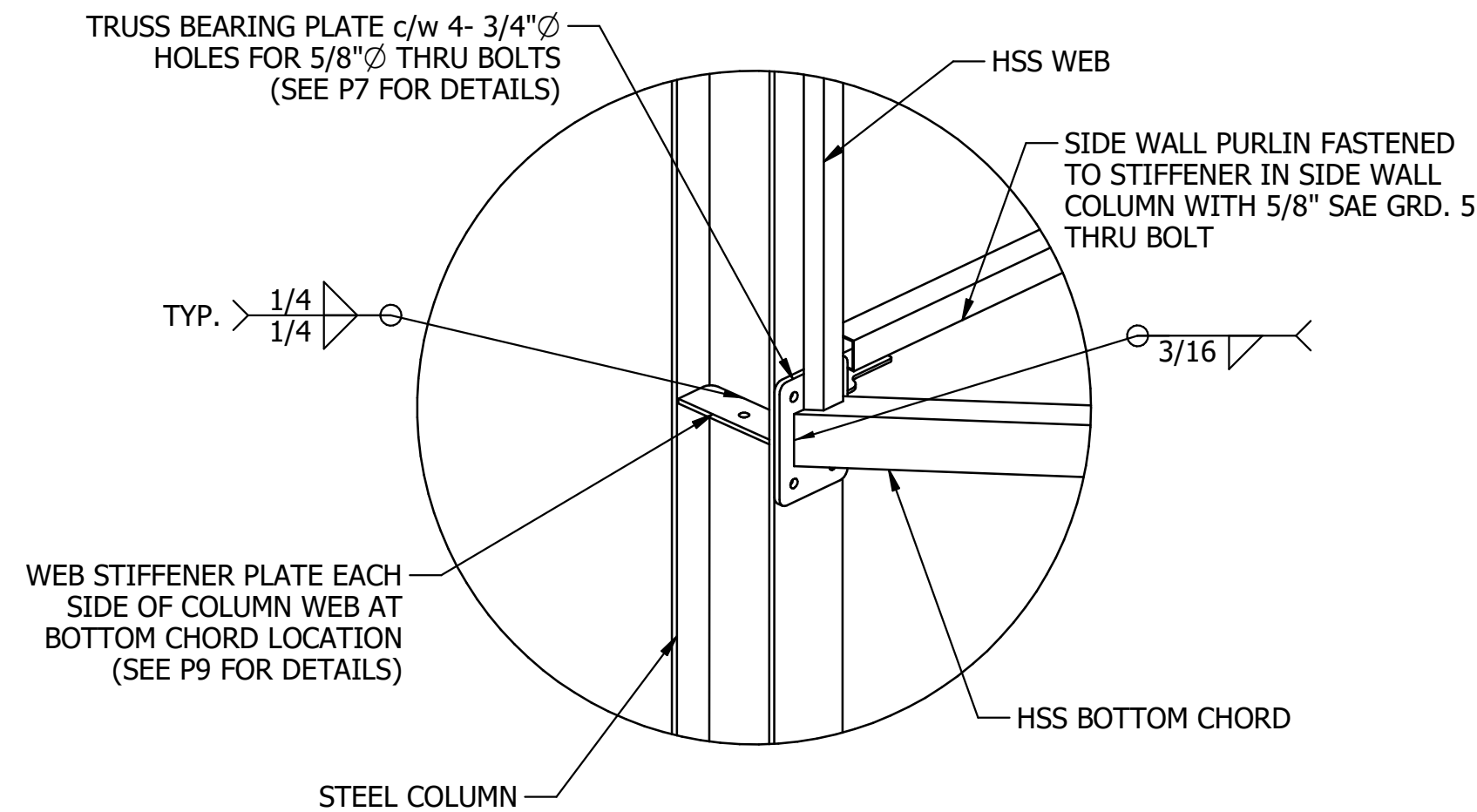
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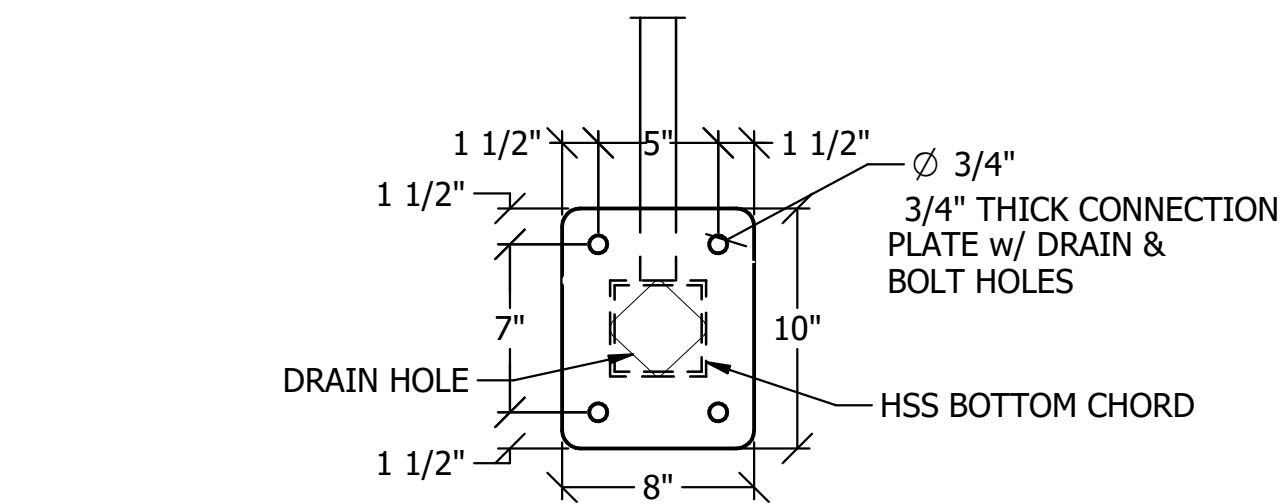
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PROJECT STATUS & VERSION:	STRUCTURAL DESIGN DRAWINGS

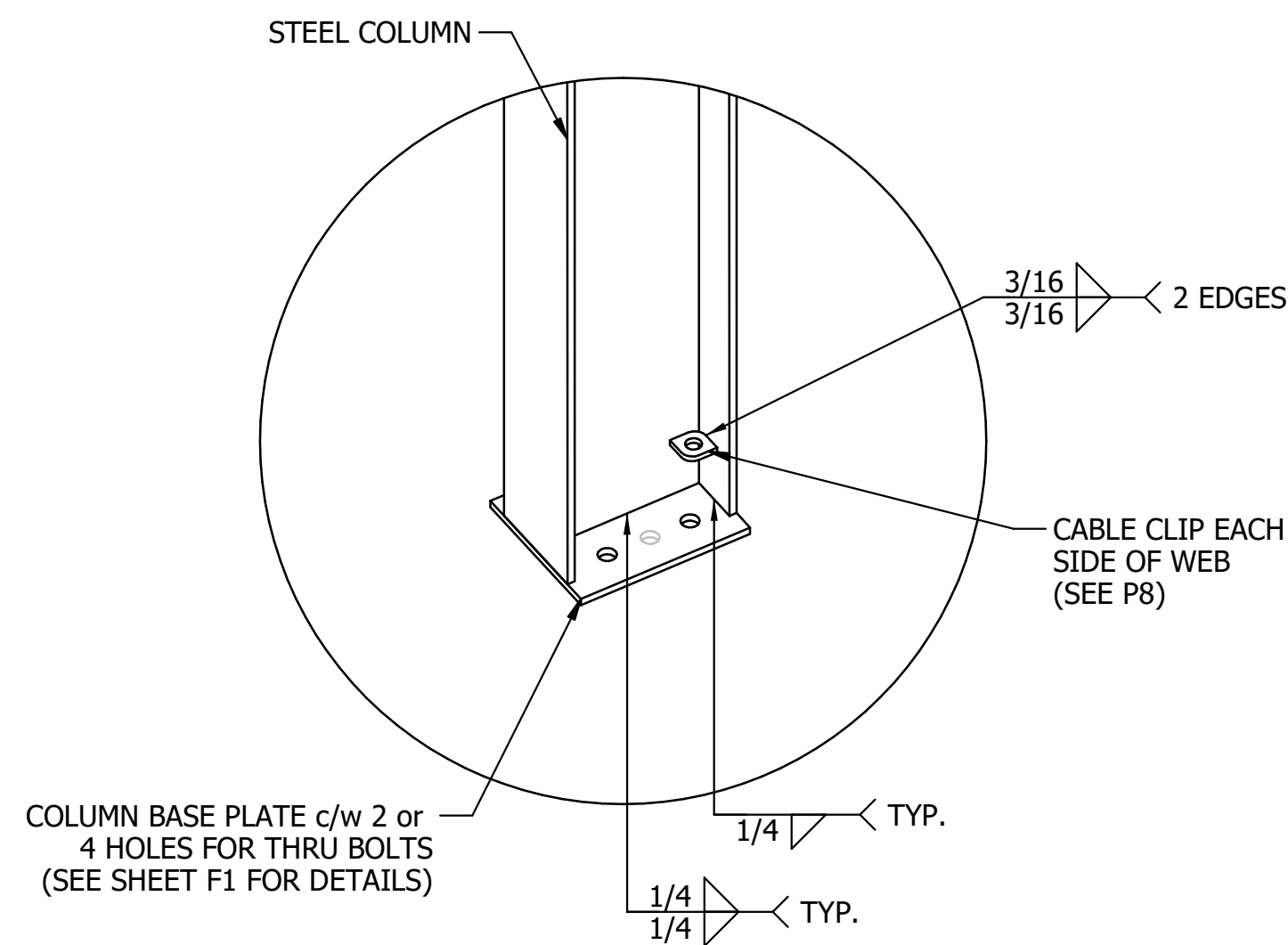
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SHEET NO.	



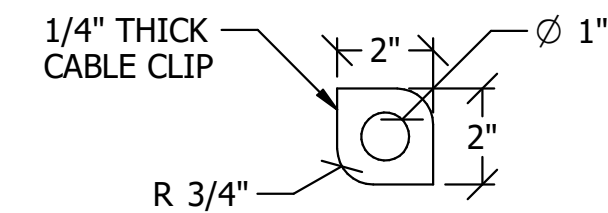
C
S6 **TRUSS HEEL DETAIL**



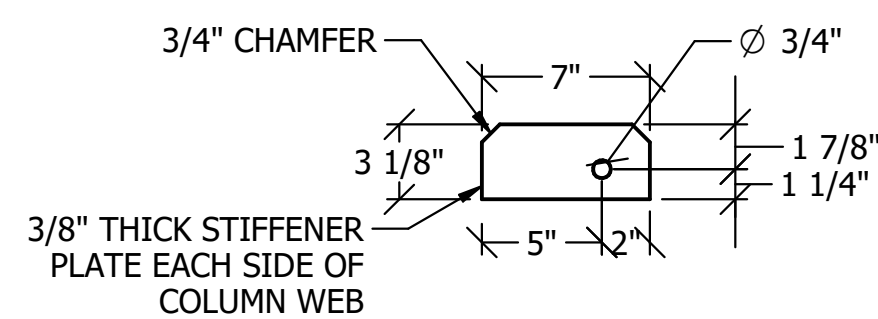
P7 **BOTTOM CHORD TO COLUMN CONNECTION PLATE**
SCALE: 1-1/2"=1'-0"



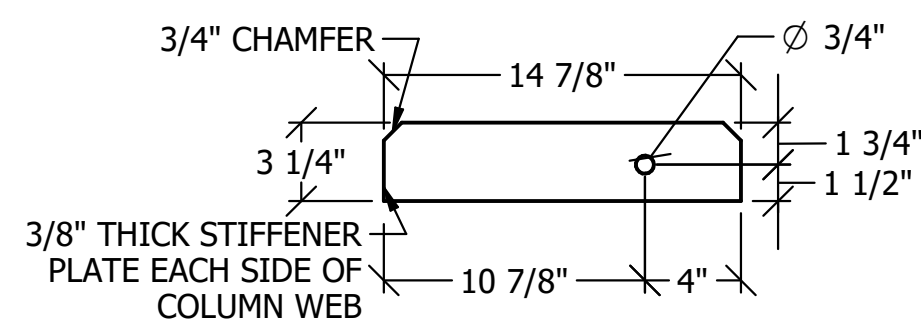
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S6 **COLUMN BASE DETAIL**



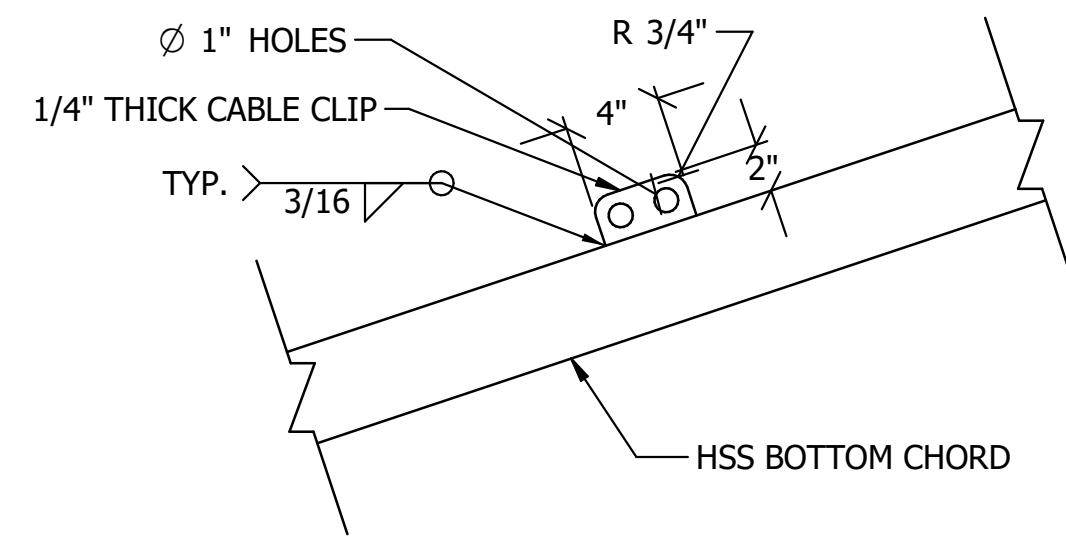
P8 **COLUMN CABLE CLIP**
SCALE: 3"=1'-0"



P9 a **COLUMN WEB STIFFENER PLATE (W8)**
SCALE: 1-1/2"=1'-0"



P9 b **COLUMN WEB STIFFENER PLATE (W16)**
SCALE: 1-1/2"=1'-0"



E
S6 **SWAY BRACING CABLE CLIP DETAIL**
SCALE: 1-1/2"=1'-0"

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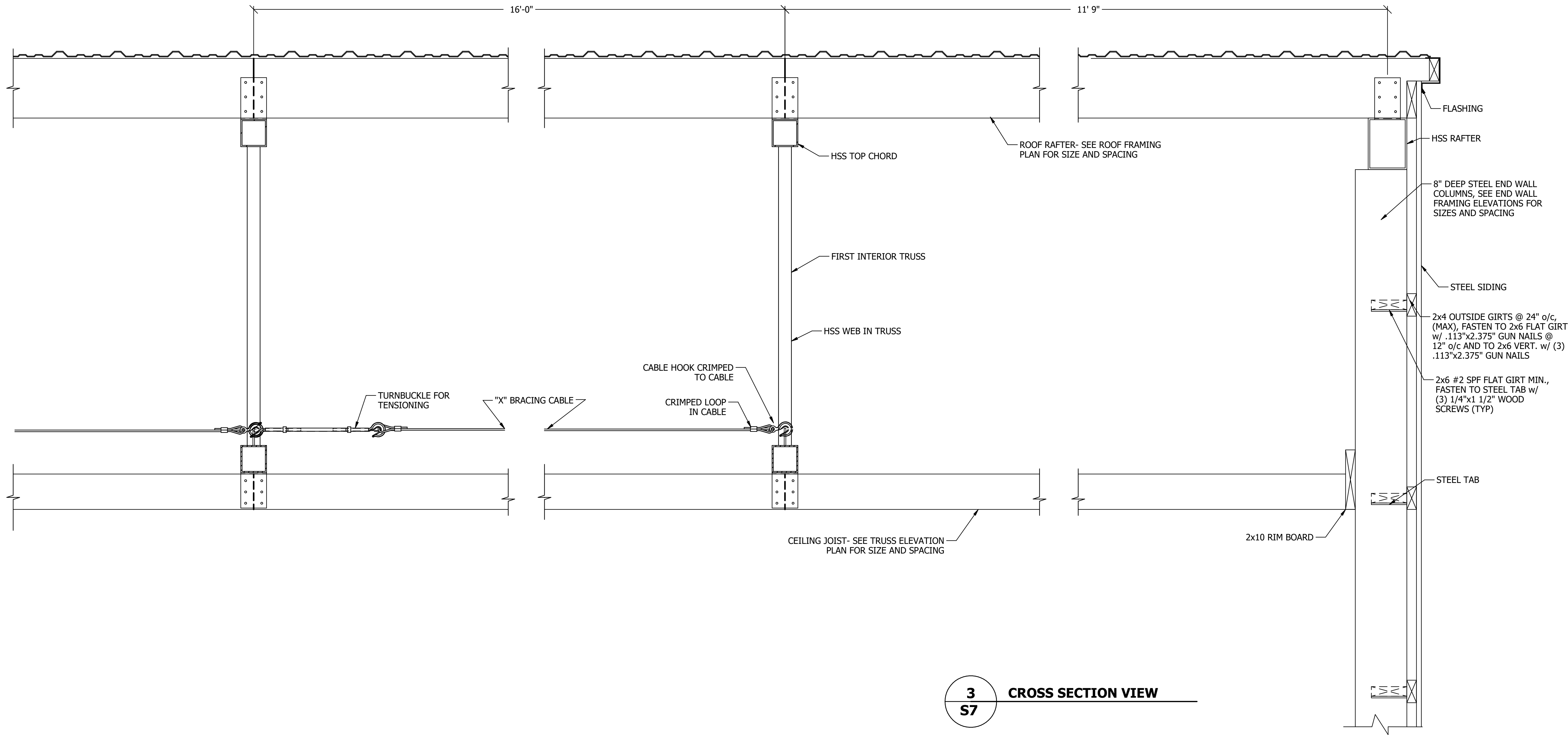
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DRAWN BY:	CHECKED BY:
CHRISTIAN	RAMA
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LICENSED PROFESSIONAL ENGINEER



WECOVER STRUCTURES INC. BEARING
CERTIFICATION OF AUTHORIZATION
#100522842 IS AUTHORIZED BY
PROFESSIONAL ENGINEERS ONTARIO

NOTE:
THIS IS FOR THE STRUCTURAL
DESIGN OF THIS BUILDING ONLY

RIDGELINE
STRUCTURAL SYSTEMS INC.

206 SOUTH MAIN STREET
KOUTS, IN 46347
(P) 219.766.0837 | (F) 219.766.0841

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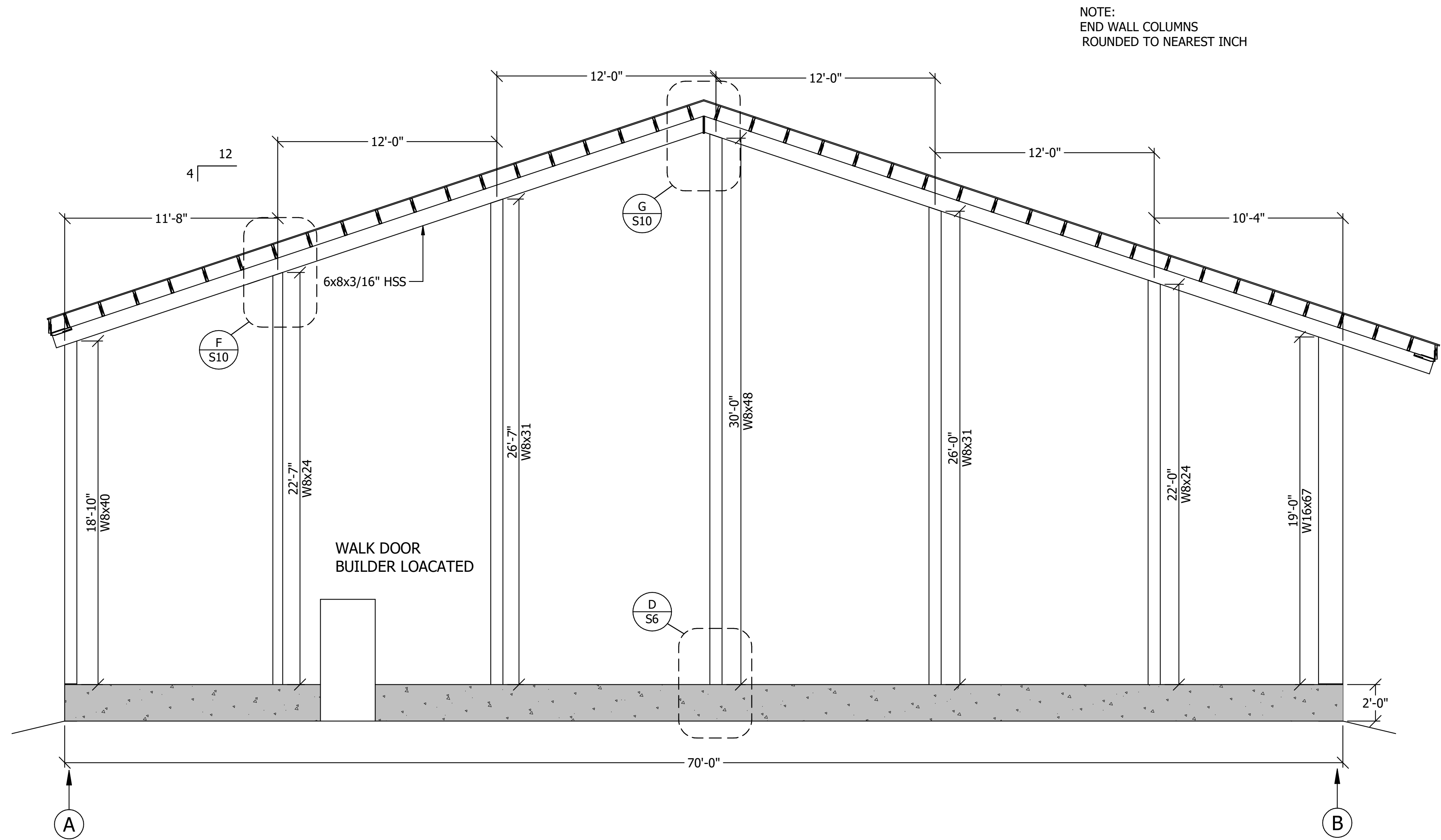
PROJECT NORTH:	TRUE NORTH:
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CONTRACTOR TO CHECK ALL DIMENSIONS
AND ELEVATIONS AND REPORT ANY DISCREPANCY
TO THE ENGINEER BEFORE PROCEEDING WITH
THE WORK

DO NOT SCALE DRAWINGS

CLIENT:
CORNERSTONE PRESSURE WASHING
PROJECT NAME:
CORNERSTONE PRESSURE WASHING
PROJECT LOCATION:
5382 LAYTON ST. WYOMING, ON N0N 1T0
PROJECT STATUS & VERSION:
STRUCTURAL DESIGN DRAWINGS

DRAWN BY:	CHECKED BY:
CHRISTIAN	RAMA
PAGE DESCRIPTION:	
PLATES AND CONNECTIONS	
PROJECT START DATE:	LAST PRINT DATE:
6/23/2021	03/16/2022
PROJECT/ CONTRACT NO.	
WECOVER 21-033	
RIDGELINE 101.21.2115-ADD2	
SHEET NO.	



4
S8

END WALL ELEVATION
SCALE: 1/4" = 1'-0"

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03/16/2022	FLOOR PLAN LAYOUT REVISED

WECOVER
STRUCTURES

22694 Victoria Rd, RR 3,
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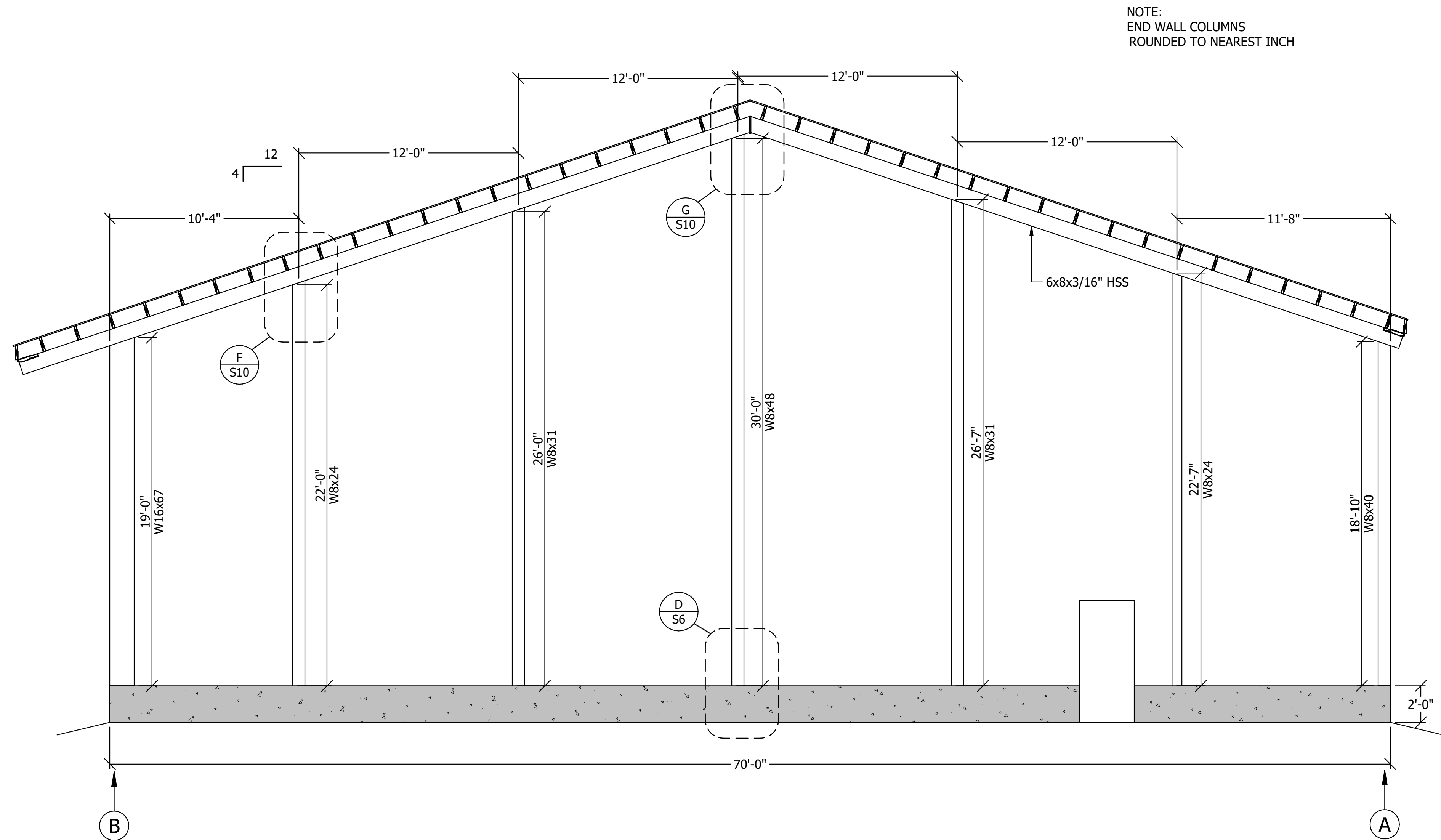
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5
S9 **END WALL ELEVATION**
SCALE: 1/4" = 1'-0"

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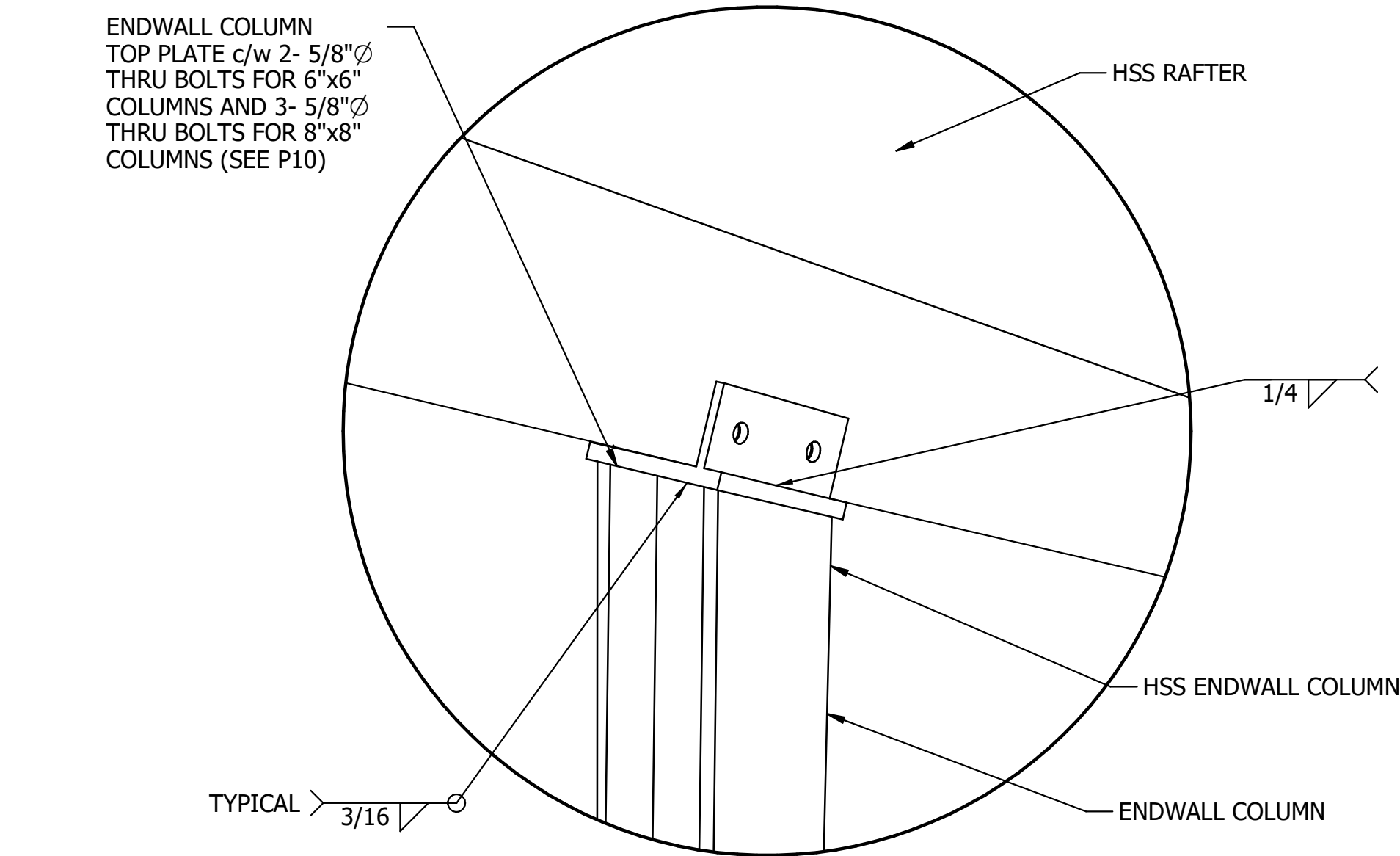
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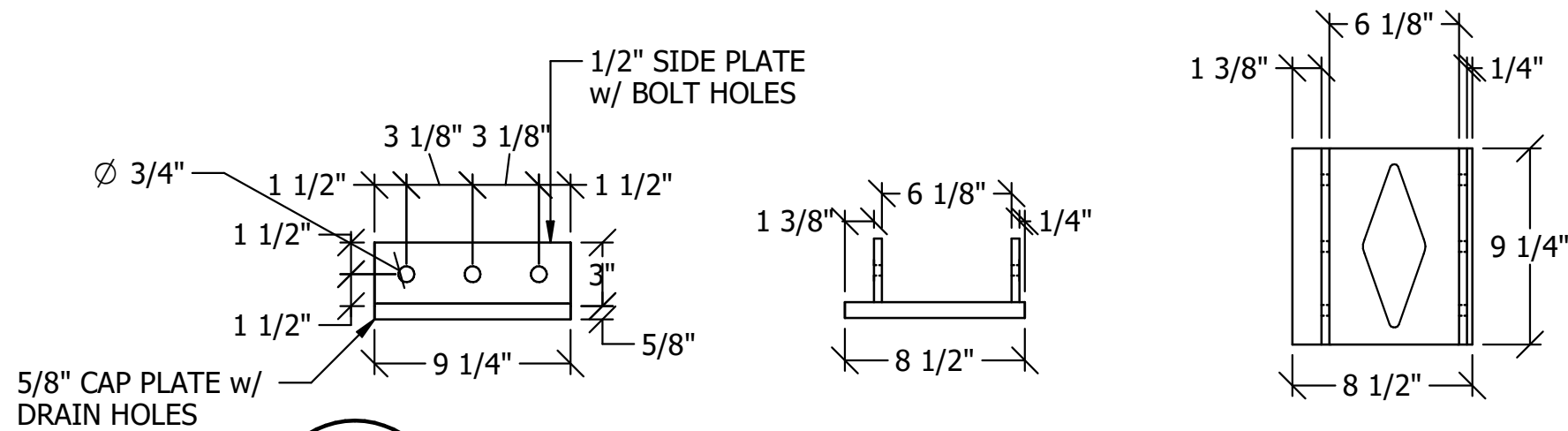
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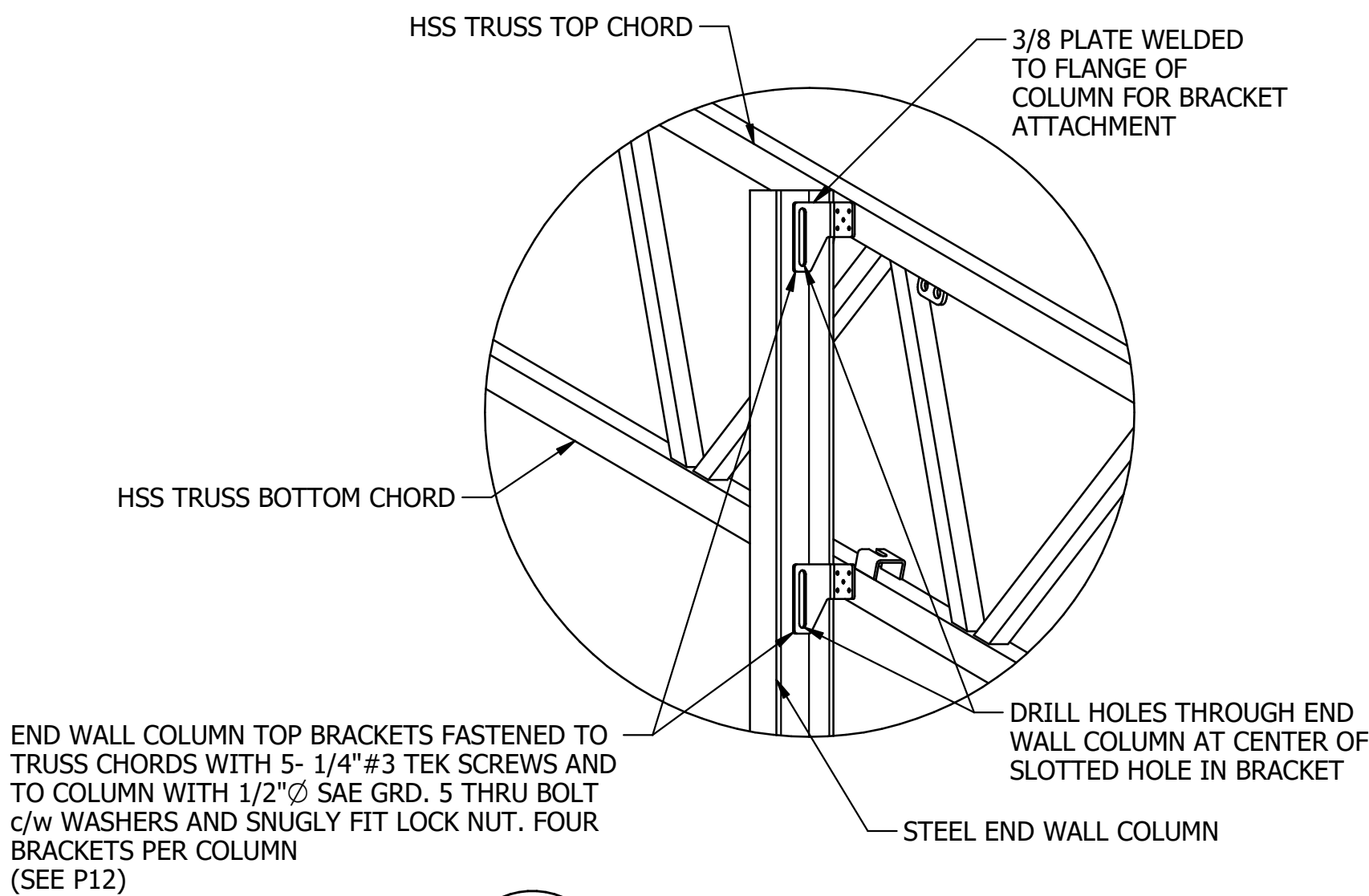
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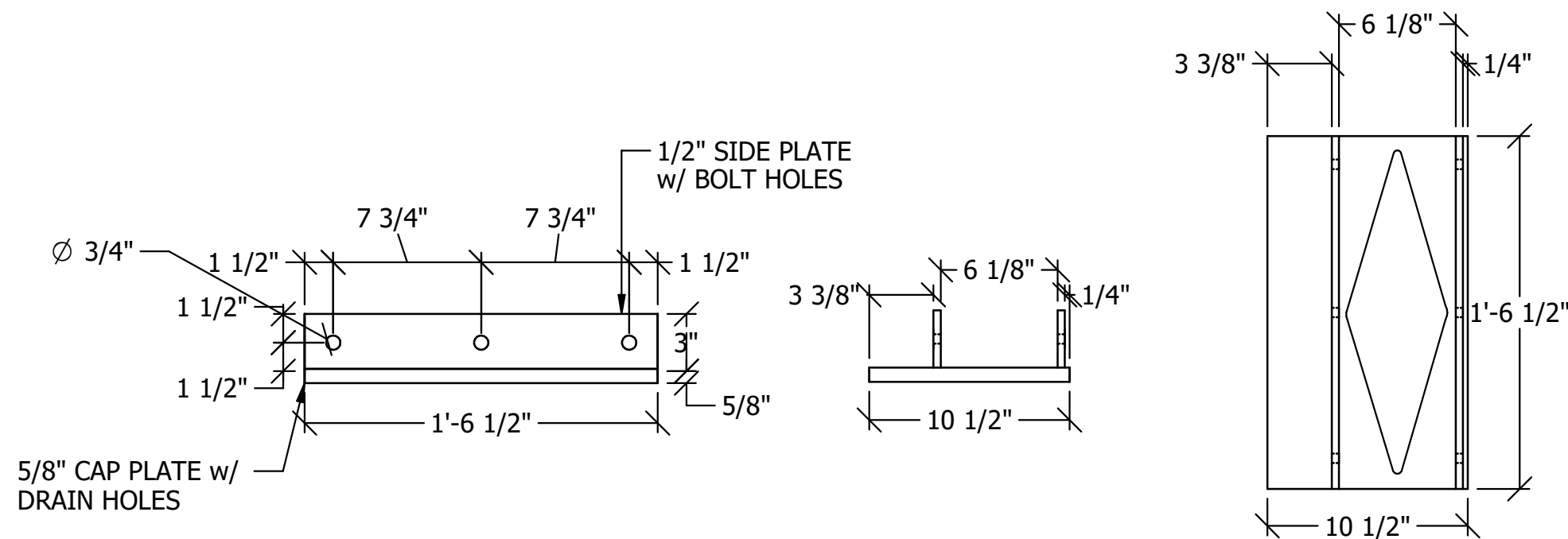
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S10 **END WALL COLUMN CONNECTION (TOP)**



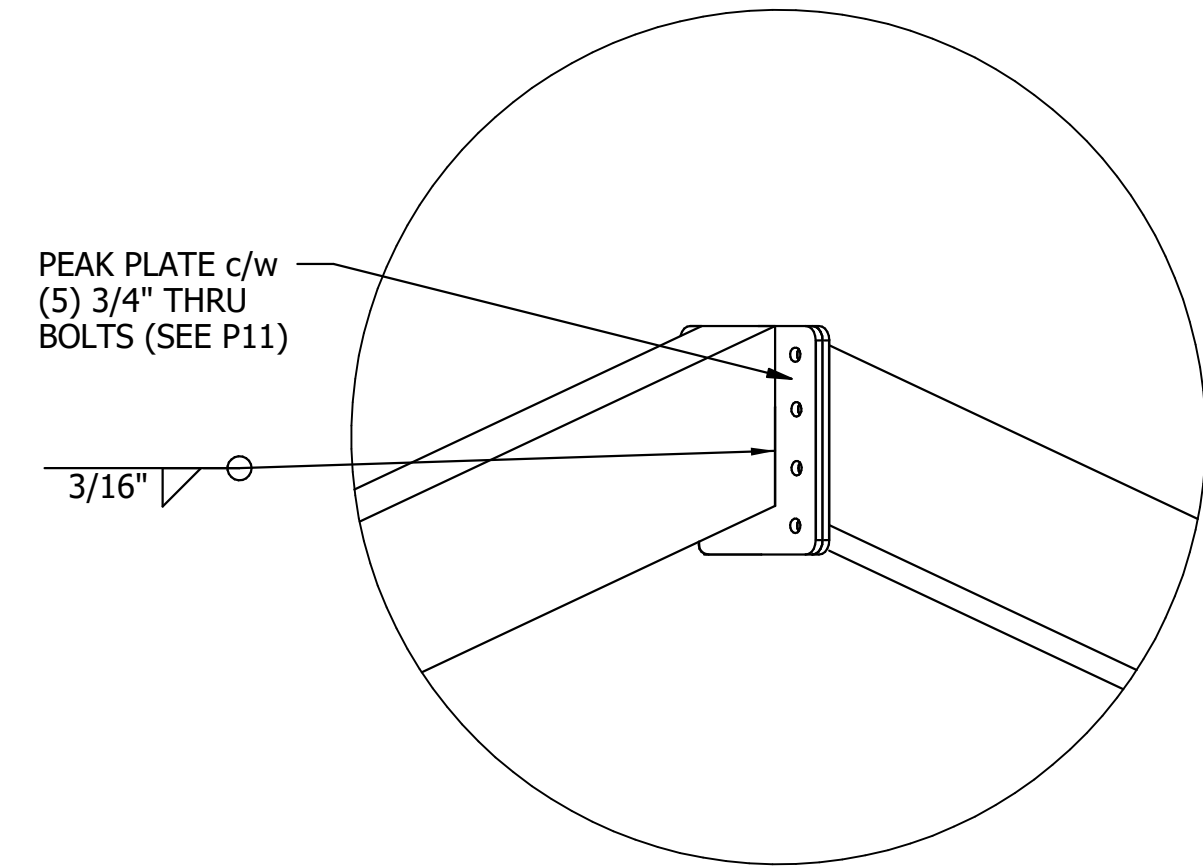
P10a **END WALL COLUMN TOP PLATE (W8 COLUMNS)**
SCALE: 1 1/2"=1'-0"



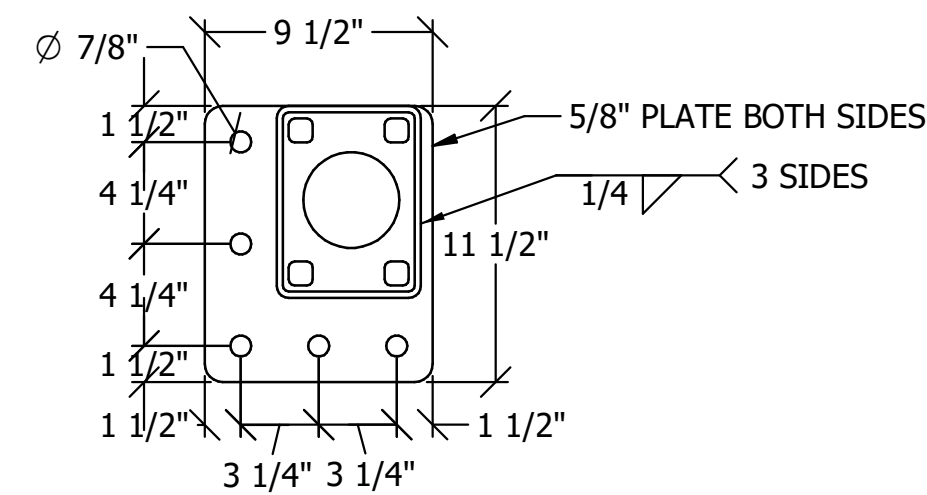
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S10 **INTERIOR COLUMN TOP DETAIL**



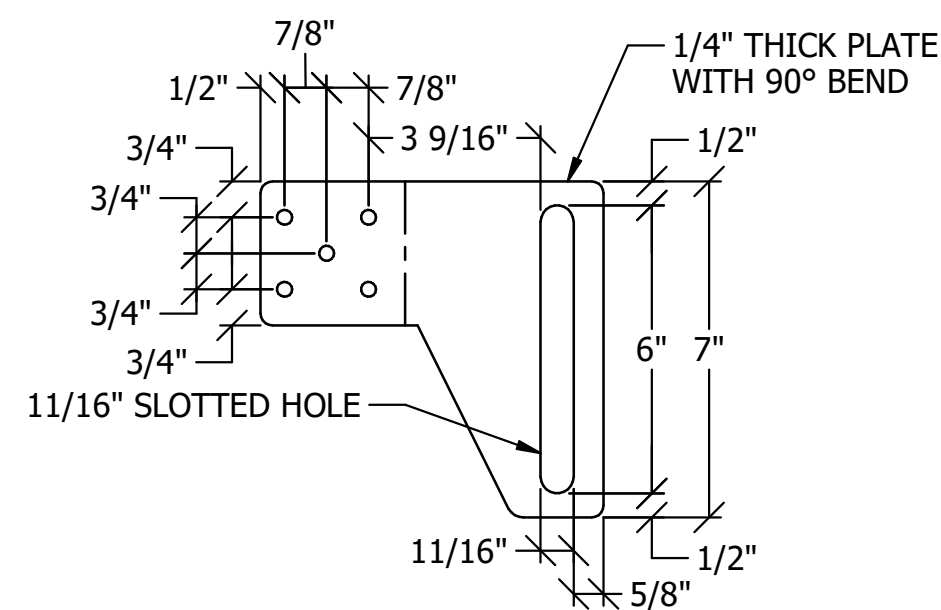
P10b **END COLUMN TOP PLATE (W16 COLUMNS)**
SCALE: 1 1/2"=1'-0"



G
S10 **PEAK PLATE CONNECTION**



P11 **ENDWALL RAFTER PEAK/SPLICE PLATE**
SCALE: 1 1/2"=1'-0"



P12 **END COLUMN TOP BRACKET**
SCALE: 3"=1'-0"

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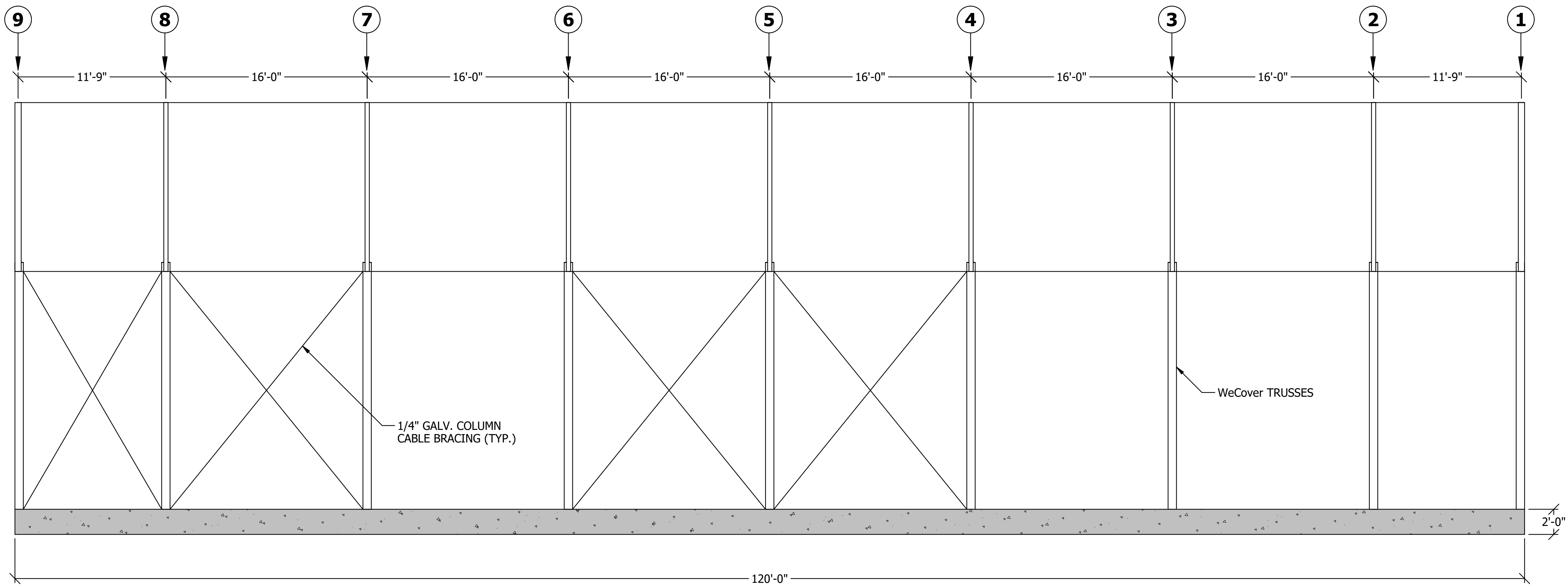
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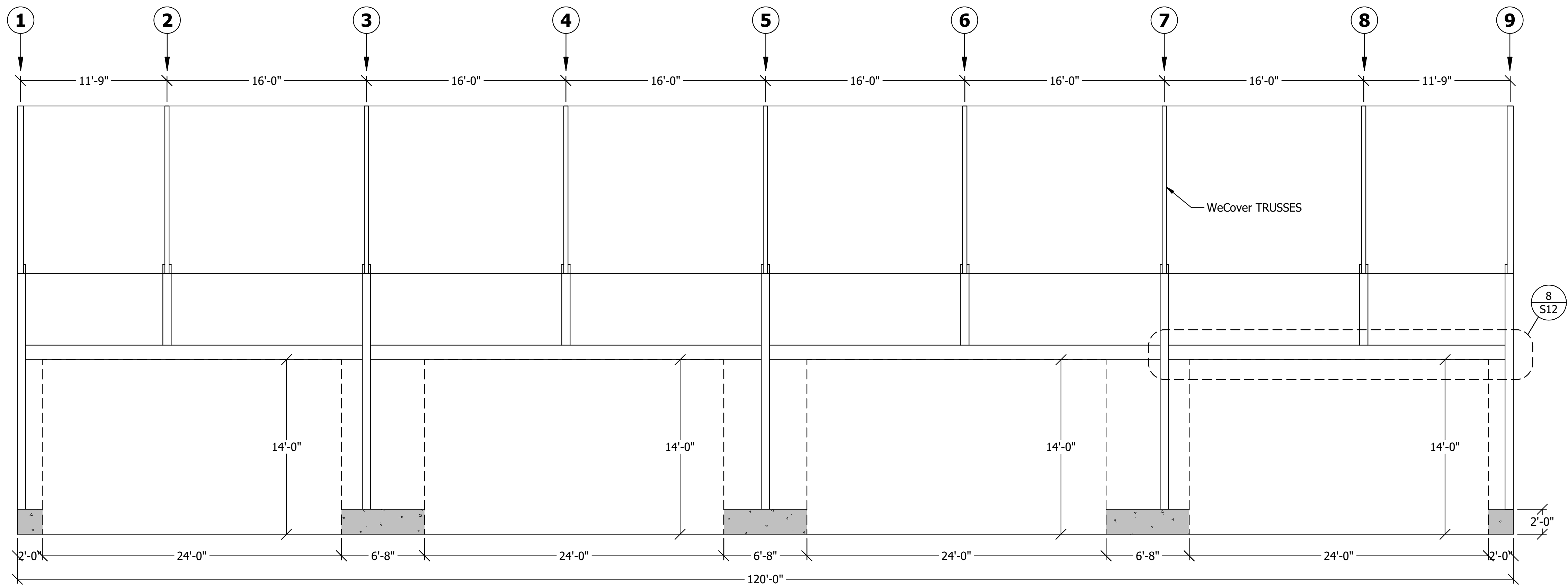
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6 **SIDE WALL A ELEVATION**
S11 **SCALE: 3/16" = 1'**

SIDE WALL ENCLOSURE:
CLADDING BY OTHERS

NOTE: ROOF BRACING NOT SHOWN FOR CLARITY



7 **SIDE WALL B ELEVATION**
S11 **SCALE: 3/16" = 1'**

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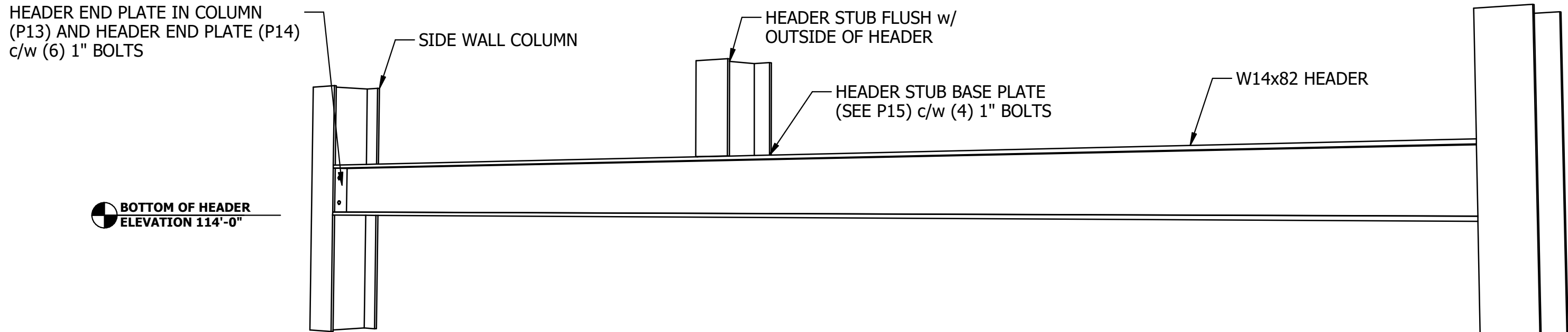
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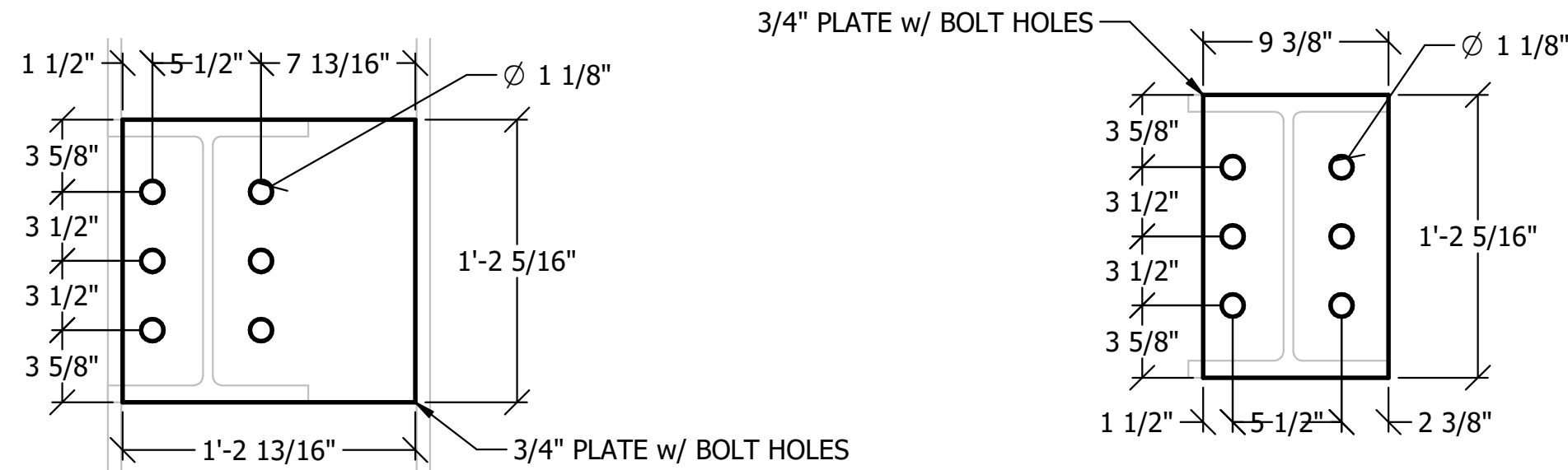
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8 SIDE WALL HEADERS

S12

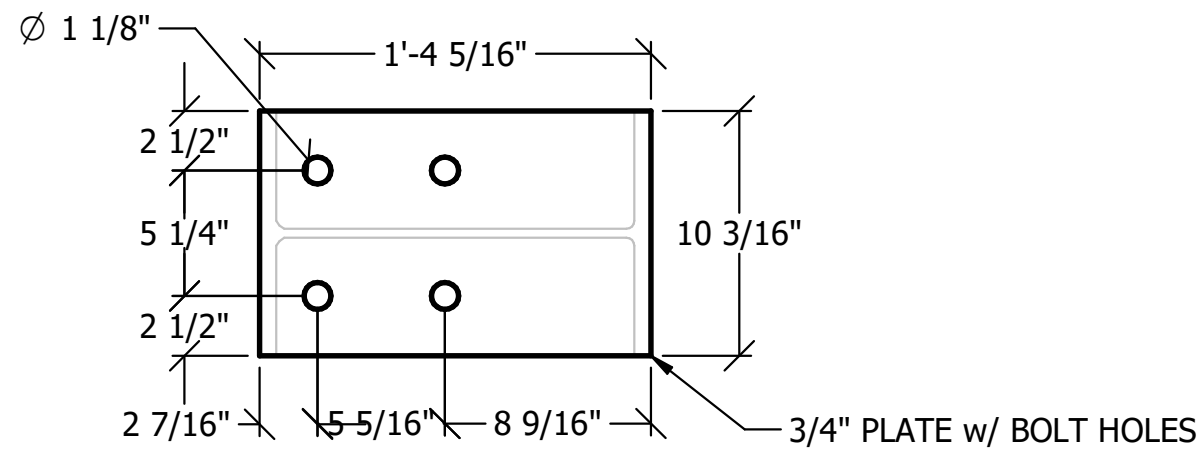


P13 HEADER END PLATE IN COLUMN

SCALE: 1-1/2"=1'-0"

P14 HEADER END PLATE

SCALE: 1-1/2"=1'-0"



P15 STUB COLUMN BASE PLATE

SCALE: 1-1/2"=1'-0"

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GENERAL NOTES

- ALL WORK SHALL COMPLY WITH ANY LOCAL AND NATIONAL CODES, LATEST EDITIONS AND OTHER LOCAL GOVERNING DOCUMENTS & APPLICABLE STANDARDS
- THESE PLANS ARE FOR STRUCTURAL DESIGN ONLY. IT IS THE RESPONSIBILITY OF THE OWNER/CONTRACTOR TO CO-ORDINATE THE DESIGN WITH RESPECT TO PLUMBING, ELECTRICAL, MECHANICAL, VENTILATION, STABLING, DRAINAGE AND SITE PREPARATION/GRADING.
- ALL SERVICES ARE TO BE LOCATED AND VERIFIED PRIOR TO EXCAVATION.
- THESE DRAWINGS ARE BASED ON INFORMATION PROVIDED BY THE CLIENT. IF DRAWINGS ARE NOT REFLECTIVE OF EXISTING CONDITIONS, THE ENGINEER IS TO BE CONTACTED IMMEDIATELY
- THE ENGINEER OF RECORD ASSUMES THAT ALL WINDOWS AND DOORS WILL BE DESIGNED TO WITHSTAND THE WIND LOADS SHOWN AND WILL REMAIN CLOSED DURING PERIODS OF SEVERE WINDS
- THE ENGINEER OF RECORD IS NOT RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF ANY EXISTING FACILITIES. THE DESIGN AND CONSTRUCTION OF THE EXISTING FACILITIES HAS NOT BEEN REVIEWED BY THE ENGINEER OF RECORD
- WHEN IN DOUBT AS TO THE INTERPRETATION OF THE DRAWINGS, THE ENGINEER IS TO BE CONTACTED.
- ALL DOORS, WINDOWS AND ROLL-UP CURTAINS MUST BE DESIGNED BY OTHERS TO SUPPORT THE SITE LOADING CONDITIONS AND ARE RELIED UPON TO BE CLOSED IN THE EVENT OF HIGH WIND.

MATERIAL SPECIFICATIONS-STRUCTURAL STEEL

- ALL STEEL SHALL BE DESIGNED, FABRICATED & ERECTED IN CONFORMANCE TO CAN/CSA-S16-09 "LIMIT STATES DESIGN OF STEEL STRUCTURES" AND CSA S367-12.
- STRUCTURAL STEEL CONFORMS TO:

ROLLED SECTIONS & PLATE	A992, ASTM A572 GRADE 50 OR CSA G40.21(350W)
HSS SECTIONS	ASTM A500
PURLINS	ASTM A500
- STRUCTURAL STEEL, ROUND AND SQUARE TUBING, HOT OR COLD FORMED AND INDUCTION WELDED TO CONFIRM THE PHYSICAL CHARACTERISTICS AND COMPOSITION OF THE SUPPLIED STEEL.

MATERIAL SPECIFICATIONS-CORROSION PROTECTION

PRODUCT	GALV. STANDARD	TYPE	COATING WEIGHT	THICKNESS
STEEL TUBE	CAN/CSA-G164	IN-LINE	0.6 OZ/SQ.FT	1.0 MILS *
STEEL PLATE & HSS	CAN/CSA-G164	HOT DIP	2.0 OZ/SQ.FT	3.4 MILS
CABLES	FEDERAL SPEC. RR-W410D			

- CHROMATE CONVERSION COATING APPLIED OVER THE GALVANIZED SURFACE COAT DEMONSTRATING THE ABILITY TO WITHSTAND A MINIMUM OF 1000 HOURS OF ACCELERATED SALT FOG TESTING TO THE 5% RED RUST CONDITION IN ACCORDANCE WITH ASTM B117.
- THE REPAIR TO THE TRUSS AT WELD LOCATIONS IS COMPLETED USING SPRAYED ZINC (METALLIZING) IN ACCORDANCE WITH ASTM A 780 - 01. SURFACES TO BE RECONDITIONED SHALL BE BLAST CLEAN, DRY AND FREE OF OIL, GREASE, CORROSION, FLUX RESIDUE AND WELD SPATTER IN ACCORDANCE WITH SSPC-SP5/NACE NO. 1 TO ENSURE THAT A SMOOTH RECONDITIONED COATING CAN BE COMPLETED. SURFACE PREPARATION SHALL BE EXTENDED INTO THE SURROUNDING UNDAMAGED GALVANIZED COATING. FIELD TOUGH UPS TO DAMAGED GALVANIZING PROTECTION IS COMPLETED USING 99.9% ZINC RICH SPRAY PAINT.
- ANTICORROSIVE ALKYD PRIMER CONFIRMING TO CAN/CGSB-1.4, IS USED AS A PRIMER FOR STRUCTURAL STEEL BY SPRAYING ON STEEL OR EASY BRUSHING 626 SQ.FT./IMP.GAL@1.0 MIL DRY FILM THICKNESS.

MATERIAL SPECIFICATIONS-GALVANIZED CABLES

- DIAGONAL STEEL CABLES AS PER FEDERAL SPEC. RR-W4100

DIAMETER INCHES	APPROX. WEIGHT (LBS/1000 FT.)	BREAK STRENGTH (LBS)
1/4	110	7000

- SHACKLES, TURNBUCKLES AND PINS TO BE GRADE 100-110 STEEL

MATERIAL SPECIFICATIONS-BOLTS

- ANCHOR BOLT TO BE DRILLED & EPOXIED INTO THE FOUNDATION WITH A MINIMUM OF 10" EMBEDMENT (U.N.O.).
- ANCHOR BOLT DIAMETERS ARE DETERMINED IN ACCORDANCE WITH CAN/CSA-S16-09 USING FY = 105 KSI (724 MPA) & Fut = 125 KSI (861 MPA).
- ANCHOR BOLT PROJECTIONS (BASED ON NO GROUT) ARE MIN. 2" AND MAX. 3 1/2".
- ALL STRUCTURAL BOLT DIAMETERS TO CONFORM TO SAE J429 GR 8 U.N.O.
- ALL STRUCTURAL NUT DIAMETERS LESS THAN 1" CONFORM TO SAE J429 GR 8 U.N.O., BE PLATED / GALVANIZED OR J5500 COATED FINISH.
- SCREWED CONNECTIONS OF STRUCTURAL STEEL SHALL BE MADE AS FOLLOWS: #14-14 SELF DRILLING SCREWS FOR HSS WALL THICKNESSES UP TO 3/16", AND #12-24 SELF DRILLING SCREWS FOR HSS WALL THICKNESSES OF 1/4" AND GREATER
- BOLTS IN CONNECTIONS NOT SUBJECT TO TENSION LOADS, OR WHERE LOOSENING DUE TO VIBRATION OR LOAD FLUCTUATIONS ARE NOT DESIGN CONSIDERATIONS NEED ONLY BE SNUG TIGHTENED TO A TIGHTNESS THAT EXISTS WHEN ALL PLIES IN A JOINT ARE IN FIRM CONTACT. BOLTS SUBJECT TO TENSION LOADS REQUIRE PRE TENSIONING TO MINIMUM VALUES SHOWN IN FOLLOWING TABLE.

BOLT LENGTH (MEASURED FROM UNDERSIDE OF THE HEAD TO THE EXTREME END OF THE POINT) UP TO AND INCLUDING 4 BOLT DIAMETERS OVER 4 DIAMETERS AND NOT EXCEEDING 6 DIAMETERS OR 8 INCHES EXCEEDING 8 DIAMETERS OR 8 INCHES	TURN
	1/3
	1/2
	2/3

NOTE: NUT ROTATION IS ROTATION RELATIVE TO A BOLT REGARDLESS WHETHER THE NUT OR BOLT IS TURNED. TOLERANCE ON ROTATION: 30° OR UNDER

MATERIAL SPECIFICATIONS-ROOF FABRIC

1. NOVA-SHIELD II WITH ARMORKOTE RU88R-6 (4 MIL) WOVEN COATED FABRIC BY ENGINEERED COATED PRODUCTS (ECP).
2. THE FABRIC IS TENSIONED BOTH VERTICALLY AND HORIZONTALLY TO PREVENT WEAR ABRASION, AND IS SECURED BY AN ALUMINUM TRACK ON TOP OF EACH TRUSS SECTION.
3. THE ROOF FABRIC (IF USED ON THIS BUILDING) IS AN IMPORTANT STRUCTURAL COMPONENT AND MUST BE REPLACED IMMEDIATELY AFTER TEAR OR DEFECT IS FOUND.
4. FABRIC TYPE USED: NON FIRE RATED

EXCAVATION AND BACKFILL

- ALL TOPSOIL AND OTHER FOREIGN MATERIAL TO BE REMOVED FROM BELOW BUILDINGS
- FOUNDATIONS HAVE BEEN DESIGNED FOR A SOIL BEARING CAPACITY OF 3000 PSF (143KPA)
- SHOULD UNUSUALLY SOFT SOILS BE ENCOUNTERED DURING EXCAVATION, NOTIFY THE ENGINEER OF RECORD BEFORE PROCEEDING WITH CONSTRUCTION. CONTRACTOR MUST NOTIFY THE ENGINEER OF ANY CONCERNS WITH REGARDS TO, BUT NOT LIMITED TO SOIL BEARING CAPACITY, SLOPE STABILITY, GROUNDWATER AND DRAINAGE.
- GRANULAR FILL UNDER THE FLOOR SLAB SHALL BE FREE-DRAINING CLEAN GRANULAR "B" MATERIAL OR BETTER, COMPACTED TO A MINIMUM 98% STANDARD PROCTOR DENSITY AT OPTIMUM MOISTURE
- GRANULAR FILL UNDER FOOTINGS SHALL BE FREE-DRAINING CLEAN GRANULAR "B" MATERIAL OR BETTER, COMPACTED TO A MINIMUM 100% STANDARD PROCTOR DENSITY AT OPTIMUM MOISTURE
- ANY OTHER FILL MATERIAL USED IS TO BE INSPECTED AND APPROVED BY A QUALIFIED GEOTECHNICAL PROFESSIONAL WITH A REPORT SUBMITTED TO THE ENGINEER OF RECORD
- ALL BACKFILL MATERIAL TO BE FREE DRAINING CLEAN GRANULAR MATERIAL. IF SUITABILITY OF BACKFILL MATERIAL IS QUESTIONABLE, THE PROJECT ENGINEER IS TO BE CONTACTED IMMEDIATELY.
- ALL FOOTINGS TO HAVE A MINIMUM OF 48" OR MORE OF COVER FOR FROST PROTECTION U.N.O.
- FINAL GRADING TO SLOPE AWAY FROM THE BUILDING

CONCRETE

- ALL CONCRETE MATERIALS AND WORKMANSHIP CONFORM TO CSA CAN3-23.1-04 AND CAN3-A23.2-04
- ALL REINFORCING STEEL SHALL BE DEFORMED AS DEFINED IN CSA F30.18 2009.
- MINIMUM RADIUS FOR BENT REBAR IS 60MM FOR 10M REBAR AND 90MM FOR 15M REBAR
- OVERLAP REBAR 24" FOR SPLICES IN CONTINUOUS REBAR LENGTHS
- WHERE REBAR JOIN AT CORNERS, PROVIDE CORNER BARS 24" EACH WAY.
- REINFORCEMENT IS TO BE LOCATED IN THE CENTER OF THE WALL, EXCEPT WHERE OTHERWISE NOTED.
- MINIMUM CONCRETE COVERAGE TO REINFORCEMENT FOR FOOTINGS SHALL NOT BE LESS THAN 3" FROM SOIL/FILL BELOW
- MINIMUM CONCRETE COVERAGE TO REINFORCEMENT FOR ALL OTHER STRUCTURAL COMPONENTS SHALL BE NOT LESS THAN 2"
- ALL CONCRETE TO HAVE A MAXIMUM 4" SLUMP. WHERE INCREASED WORKABILITY IS REQUIRED, PLASTICIZER IS TO BE ADDED. WATER IS NOT TO BE ADDED ON SITE.
- ALL STRUCTURAL CONCRETE AND CONCRETE EXPOSED TO FREEZE/THAW TO BE 6% AIR ENTRAINED
- WHERE APPROPRIATE, USE VIBRATION EQUIPMENT TO PLACE CONCRETE.
- ADEQUATE PROTECTION FROM FREEZING MUST BE PROVIDED TO POURED CONCRETE DURING COLD WEATHER PLACEMENT.
- ALL SLEEVES TO BE LOCATED BY ELECTRICAL AND MECHANICAL DESIGNERS PRIOR TO POURING CONCRETE.
- ALL FOUNDATION WALLS TO BE TYPE A-2 CONCRETE, 32 MPA AT 28 DAYS, WITH 5-8% AIR AND A MAX 0.45 WATER/ CEMENT RATIO.
- ALL FOOTINGS TO BE TYPE N CONCRETE, 20 MPA AT 28 DAYS
- ALL CONCRETE POURED TO CLASS OF CONCRETE SPECIFIED IN ENGINEERED DRAWINGS. ALL CONCRETE COMPONENTS NOT SPECIFIED SHALL BE CLASSED A-4. SEE FOLLOWING FOR CONCRETE CLASS SPECIFICATIONS.

CLASS OF CONCRETE	MAX W/CM	MIN. 28 DAY STRENGTH
A-1	0.40	35 MPa
A-2	0.45	32 MPa
A-3	0.50	30 MPa
A-4	0.55	25 MPa

GENERAL REVIEW

- IT IS THE RESPONSIBILITY OF THE OWNER/CONTRACTOR TO CONTACT THE MUNICIPALITY FOR INSPECTIONS PERFORMED BY THE LOCAL BUILDING DEPARTMENT.
- THE ENGINEER OF RECORD REQUIRES THAT THE FOLLOWING COMPONENTS BE INSPECTED:
 - SOIL CONDITIONS. WHEN THE SITE HAS BEEN COMPLETELY PREPARED FOR CONSTRUCTION, THE ENGINEER MUST BE NOTIFIED TO PROVIDE AN INSPECTION OF THE SOIL CONDITIONS.
 - FOOTINGS. WHEN THE CONCRETE FORMWORK AND REINFORCING STEEL HAVE BEEN SET FOR THE PLACEMENT OF THE FOOTINGS.
 - CONCRETE REINFORCEMENT. WHEN THE REINFORCING STEEL HAS BEEN TIED FOR CONCRETE COMPONENTS. NOTE THAT AS PART OF A GENERAL REVIEW, IT IS NOT REASONABLE FOR THE ENGINEER TO REVIEW THE REINFORCEMENT EACH TIME THAT CONCRETE IS POURED. THE CONTRACTOR ASSUMES ALL RESPONSIBILITY FOR PROVIDING THE PROPER REINFORCEMENT AND PLACEMENT, AS SPECIFIED IN THE ENGINEERED PLANS, FOR COMPONENTS WHICH ARE NOT REVIEWED BY THE ENGINEER.
 - COMPLETION OF ERECTION. WHEN THE FACILITY HAS BEEN COMPLETELY ERECTED, PRIOR TO INSTALLING INTERIOR OR EXTERIOR SHEATHING THAT WOULD PREVENT A VISUAL INSPECTION OF KEY FRAMING COMPONENTS AND WHEN THE TRUSSES HAVE BEEN SET AND ALL OF THE PERMINENT BRACING INSTALLED AS PER DRAWING SPECIFICATIONS.
 - FINAL REVIEW. WHEN ALL STRUCTURAL COMPONENTS OF FACILITY HAVE BEEN COMPLETED.
- THE CLIENT MUST REQUEST ADDITIONAL INSPECTIONS BE PERFORMED BY THE ENGINEER IF THERE IS ANY CONCERN ABOUT OR CHANGES TO ANY COMPONENT OF THE FACILITY, FAILURE TO NOTIFY THE ENGINEER IN SUCH SITUATIONS RELEASES WECOVER STRUCTURES OF LIABILITY FOR SUCH CHANGES OR COMPONENTS.

BUILDING ERECTION

- THE CONTRACTOR/ERECTOR OF THIS STRUCTURE IS RESPONSIBLE FOR QUALITY WORKMANSHIP WHILE ERECTING THIS STRUCTURE ACCORDING TO THE DRAWINGS AND DETAILS PROVIDED AS WELL AS THE INDUSTRY STANDARDS PERTAINING TO PROPER ERECTION AND TEMPORARY BRACING, THE ENGINEER OF RECORD MAY NOT BE HELD LIABLE FOR ERRORS, OMISSIONS, OR DAMAGES INCURRED IN THE ERECTION PROCESS.
- THE ERECTOR MUST PROVIDE A SAFE WORKING ENVIRONMENT IN ACCORDANCE TO ALL LOCAL SAFETY REGULATIONS.
- ALL LIFTING DEVICES ARE TO BE DESIGNED TO LIFT THE BUILDING COMPONENTS IN A MANNER NOT TO CAUSE DAMAGE OR DEFORMATION OF THE STRUCTURE.
- THIS BUILDING IS NOT DESIGNED TO BE LIFTED AS AN ASSEMBLED UNIT.
- THE ENGINEER OF RECORD IS NOT RESPONSIBLE FOR LOSSES AND/OR DAMAGE AS A RESULT OF LIFTING THIS BUILDING. IF, HOWEVER, IT HAS BEEN DETERMINED TO LIFT THIS BUILDING AS A UNIT IT IS THE RESPONSIBILITY OF THE PERSON, FIRM, OR COMPANY CONTRACTED TO LIFT THE BUILDING TO ENSURE THE LIFT DOES NOT DAMAGE THE BUILDING.
- TO ERECT THIS STRUCTURE ASSEMBLE FRAMES FROM TRUSS COMPONENTS, THEN STARTING AT A BRACED BAY, ERECT AND SUPPORT FRAMES USING TEMPORARY BRACING AS REQUIRED TO ENSURE STABILITY OF THE FRAMES.
- INSTALL PURLINS AND CROSS BRACING. PLUMB AND SQUARE FRAMES IN ACCORDANCE WITH CAN/CSA-16 AND OSHA 29 CFR PART 1926 - SAFETY STANDARDS FOR STEEL ERECTION.
- STRUCTURAL FRAMES ARE CONSIDERED PLUMB, LEVEL AND ALIGNED WHEN THE VARIANCE DOES NOT EXCEED 1: 500. ENSURE ALL PURLINS REMAIN PARALLEL.
- PURLINS ARE NOT DESIGNED TO CARRY ADDITIONAL HANGING LOADS. LOADS HUNG FROM PURLINS MUST BE REVIEWED AND APPROVED IN WRITING BY CERTIFYING ENGINEER. SEVERE DAMAGE TO THE BUILDING AND ACCESSORIES MAY RESULT FROM FAILURE TO COMPLY WITH THIS REQUIREMENT.
- FINISHED FLOOR ELEVATIONS AND UNDERSIDE OF BASE PLATE IS 100'-0" UNLESS NOTED.
- HOLES REQUIRED IN HSS COLUMNS, HEADERS OR PURLINS FOR FRAMED OPENINGS, DOOR OR WINDOW POST CONNECTIONS TO BE BY THE ERECTOR.
- ALL CONNECTORS USED FOR ACQ OR CA TREATED WOOD SHOULD BE GALVANIZED STEEL AS PER ASTM A653. ALL FASTENERS FOR ACQ OR CA TREATED WOOD SHOULD BE GALVANIZED IN ACCORDANCE WITH ASTM A153.
- PROVIDE ACCESS TO EACH ATTIC SPACE AS PER NBC REQUIREMENTS.

DELIVERY AND STORAGE

- THE ENGINEER OF RECORD MAY NOT BE HELD LIABLE FOR MATERIALS WHICH ARE NOT PROPERLY STORED ONCE THEY HAVE BEEN DELIVERED TO SITE.
- GALVANIZED, ALUMINUM AND COLORED MATERIALS ARE SUBJECT TO CORROSION AND DISCOLORATION IF THEY ARE IMPROPERLY STORED.
- SHORT TERM STORAGE OF COMPONENTS MAY BE TOLERATED AT THE JOB SITE PROVIDED CARE IS TAKEN TO KEEP THE MATERIALS DRY AT ALL TIMES. WHEN TRUSSES ARE TO BE STORED OUTDOORS, THEY SHOULD BE PLACED AT AN ANGLE SUFFICIENT TO PROMOTE GOOD DRAINAGE. IN ADDITION, SEVERAL INCHES OF CLEARANCE MUST BE PROVIDED BETWEEN THE LOWER END AND THE GROUND TO ALLOW VENTILATION.

FRAMING & BRACING

- ALL LUMBER TO BE SPF NO.2 OR BETTER, UNLESS OTHERWISE NOTED
- LUMBER IN CONTACT WITH THE EARTH, CONCRETE OR EXPOSED TO WEATHER ELEMENTS TO BE PRESSURE TREATED IN CONFORMANCE WITH CAN/CSA-808-M97
- ALL CONNECTORS USED FOR ACQ OR CA TREATED WOOD SHOULD BE GALVANIZED STEEL AS PER ASTM A653.

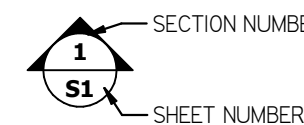
METAL CLADDING

1. STEEL SIDING AND ROOFING PANELS SHALL BE FABRICATED FROM 29 GAUGE, GRADE (80ksi) STRUCTURAL QUALITY STEEL WITH HOT DIPPED GALVANIZED COATING CONFORMING TO ASTM A653/A653M OR WITH AN ALUMINUM-ZINC ALLOY COATING CONFORMING TO ASTM A792/A792M (PLAIN PRODUCTS ONLY).
2. ALUMINUM SIDING AND ROOFING PANELS SHALL BE FABRICATED FROM NOMINAL 0.018 INCHES THICK, 3104 ALLOY,H38 TEMPER, HIGH STRENGTH ALUMINUM CONFORMING TO ASTM B209.
3. PAINT FINISH: ALL PANELS, WHEN REQUIRED, SHALL RECEIVE A FACTORY APPLIED POLYESTER COATING CONFORMING TO THE MANUFACTURER'S SPECIFICATIONS.
4. FLASHINGS SHALL BE SHOP FABRICATED FROM MATERIAL THAT IS THE SAME GAUGE AND FINISH AS THE WALL/ROOF PANELS TO WHICH THEY ARE ATTACHED. WHERE PRACTICABLE, FLASHINGS SHALL BE FURNISHED IN MAXIMUM 10ft. LENGTHS.
5. CAULKING SEALANT SHALL BE A GUN GRADE AR TAPE TYPE NON-HARDENING LONG-LIFE BUTYL (FOR SEALING METAL-TO-METAL GASKET JOINTS), OR GUN GRADE NON-ACETIC ACID CURED LONG-LIFE SILICONE (FOR FILLING VOIDS). ALL CAULKING OR SEALANT SHALL BE APPLIED IN A NEAT MANNER WITH EXCESS CAULKING OR SEALANT REMOVED FROM EXPOSED SURFACES.
6. FASTENERS: ALL SCREW FASTENERS SHALL HAVE A COMBINATION STEEL AND NEOPRENE WASHER. NAILS SHALL HAVE A FABROSEAL WASHER OR EQUIVALENT. FASTENERS SELECTION AND INSTALLATION SHALL BE AS RECOMMENDED BY CLADDING MANUFACTURER.
7. INSTALLATION OF ALL METAL CLADDING, AT FLASHING, TRIM, SEALANT AND FASTENERS SHALL BE ACCORDING TO MFR. SPECS.

LEGEND OF ACRONYMS & ABBREVIATIONS

φ	-DIAMETER	HORIZ.	-HORIZONTAL
@	-USE FOR "AND" WHEN APPROPRIATE	HSS	-HOLLOW STRUCTURAL SECTION
&	-USE FOR "AT" WHEN APPROPRIATE	HVAC	-HEATING/VENTILATION/AIR CONDITIONING
ℓ	-CENTER LINE	I.F.	-INSIDE FACE
ADDL.	-ADDITIONAL	IN.	-INCH
A.F.F.	-ABOVE FINISHED FLOOR	INSUL.	-INSULATION
ALUM.	-ALUMINUM	INT.	-INTERIOR
ARCH.	-ARCHITECTURAL	L.	-LENGTH
ASD	-ALLOWABLE STRESS DESIGN	L.L.	-LONG LEG HORIZONTAL
BL	-BUILDING LINE	L.L.V.	-LONG LEG VERTICAL
BLDG.	-BUILDING	LRFD	-LOAD AND RESISTANCE FACTOR DESIGN
BLK.	-BLOCK	MAX.	-MAXIMUM
BM	-BEAM	MFR.	-MANUFACTURER
B.O.F.	-BOTTOM OF FOOTING	MIN.	-MINIMUM
BRG.	-BEARING	MISC.	-MISCELLANEOUS
BTWN.	-BETWEEN	N.I.C.	-NOT IN CONTRACT
c/c	-CENTER TO CENTER	NO.	-NUMBER
C.I.P	-CAST-IN-PLACE	N.T.S.	-NOT TO SCALE
CL	-CENTERLINE	o/c	-ON CENTER
CLG.	-CEILING	O.F.	-OUTSIDE FACE
CLR.	-CLEAR	P.C.F.	-POUNDS PER CUBIC FOOT
CMU	-CONCRETE MASONRY UNIT	PL.	-PLATE
COL.	-COLUMN	PLF	-POUNDS PER LINEAL FOOT
CONC.	-CONCRETE	PNT.	-PAINT
CONT.	-CONTINUOUS	PWD.	-POWDER
C/W	-COMPLETE WITH	PSF	-POUNDS PER SQUARE FOOT
D. OR DP.	-DEEP	PSI	-POUNDS PER SQUARE INCH
d _s	-BAR DIAMETER	RAD.	-RADIUS
DIA.	-DIAMETER (USE φ WHEN POSSIBLE)	REQ.	-REQUIRED
DIM.	-DIMENSION	REINF.	-REINFORCING
DN.	-DOWN	RET.	-RETURN
DWG.	-DRAWING	SC	-SLIP CRITICAL
EA.	-EACH	SECT.	-SECTION
E.F.	-EACH FACE	SIM	-SIMILAR
ENG.	-ENGINEER(S)/ENGINEERING	SLR.	-SEALER
ELEC.	-ELECTRIC/ELECTRICAL	SPC.	-SPACE
ELEV.	-ELEVATION	S.S.	-STAINLESS STEEL
EQ.	-EQUAL	STL.	-STEEL
EQUIP.	-EQUIPMENT	T.B.B	-TOP & BOTTOM
E.W.	-EACH WAY	THK	-THICK
EX.	-EXISTING	T.O.F.	-TOP OF FOOTING
EXT.	-EXTERIOR	T.O.S.	-TOP OF STEEL
F.F.	-FINISHED FLOOR	T.O.W.	-TOP OF WALL
F.G.	-FINISHED GRADE	TYP.	-TYPICAL
FIN.	-FINISH(ED)	U.N.O.	-UNLESS NOTED OTHERWISE
FLR.	-FLOOR	VAR.	-VARIES
FT.	-FOOT	VERT.	-VERTICAL
FTG.	-FOOTING	W.	-WIDE/ WIDTH
GA.	-GAGE OR GAUGE	W/	-WITH
GALV.	-GALVANIZED	WD.	-WOOD
GYP.	-GYPSUM	W.W.R.	-WELDED WIRE REINFORCING
GPOW.	-GYPSUM DRYWALL		
GRD.	-GRADE		
HGT.	-HEIGHT		

SECTION MARKER LEGEND



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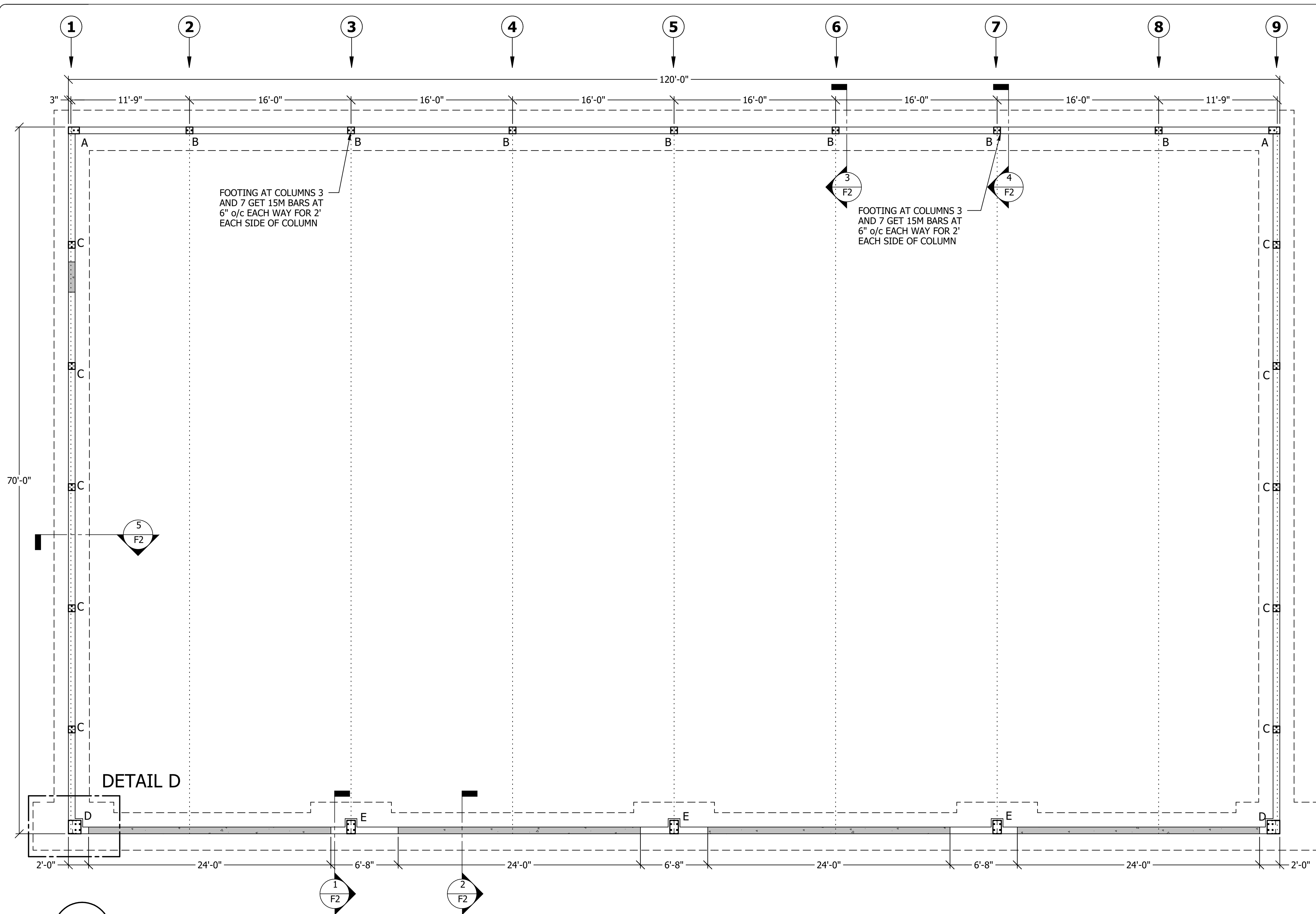
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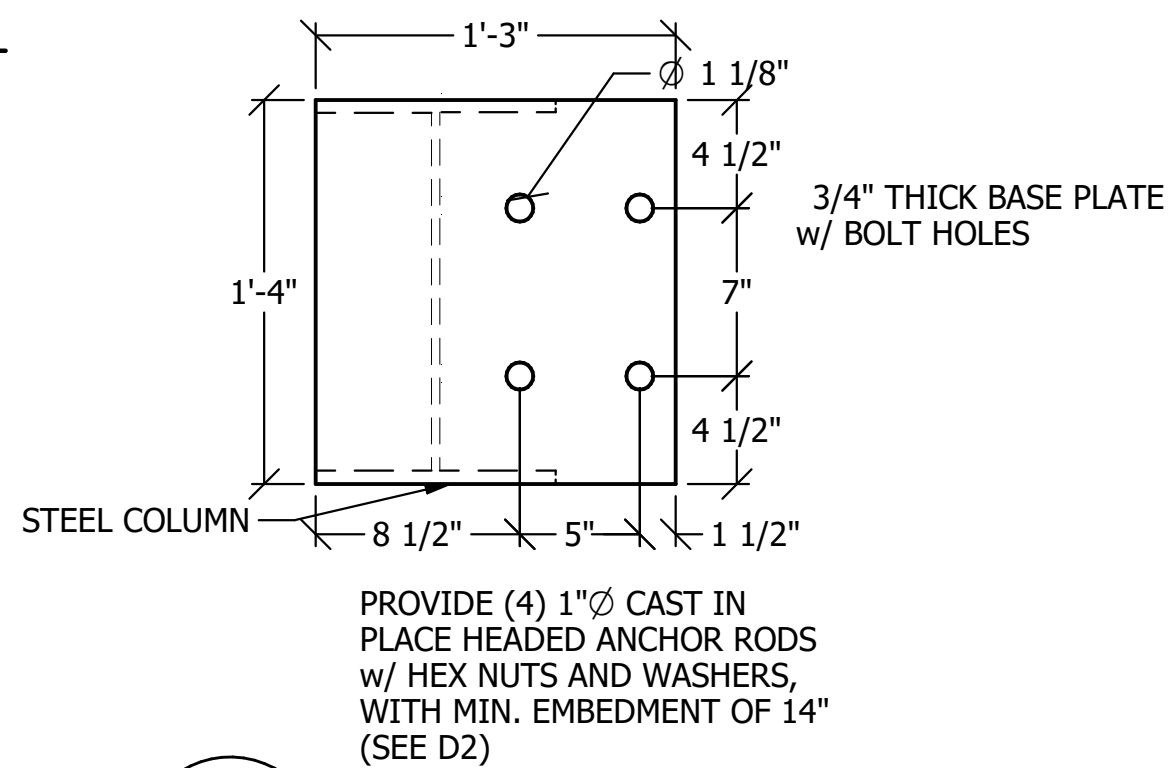
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PROJECT LOCATION:	5382 LAYTON ST. WYOMING, ON NON 1T0
PROJECT STATUS & VERSION:	STRUCTURAL DESIGN DRAWINGS

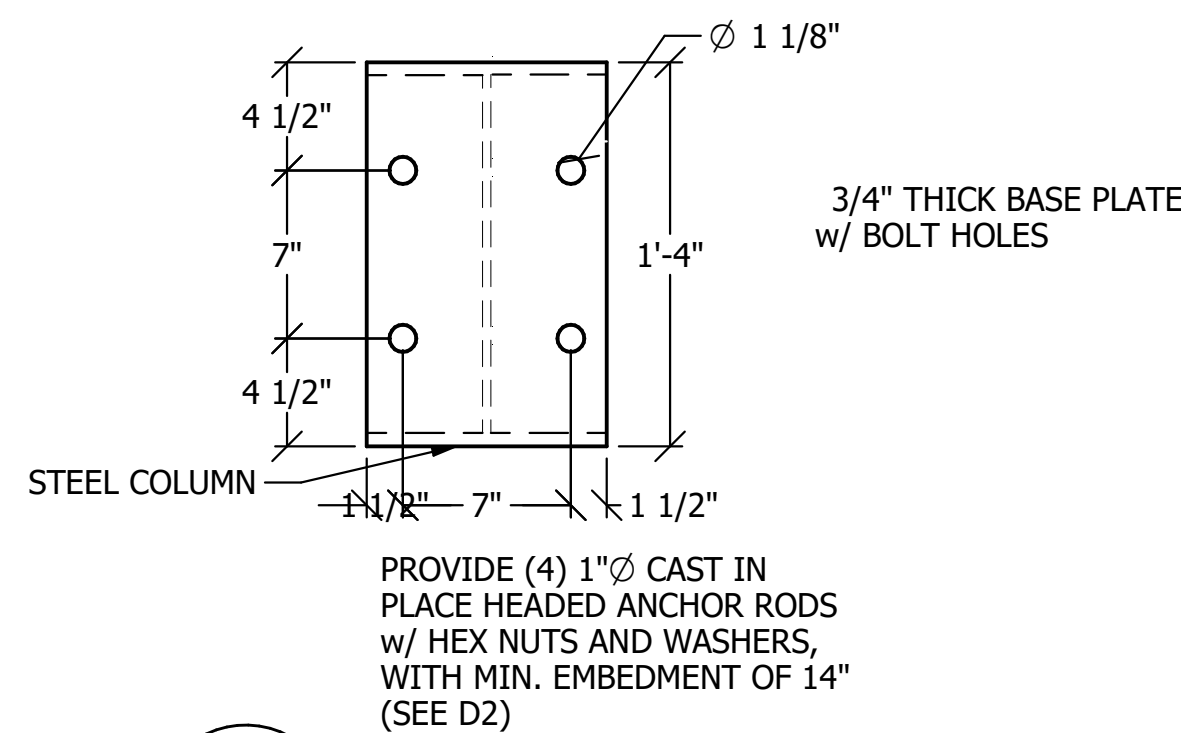
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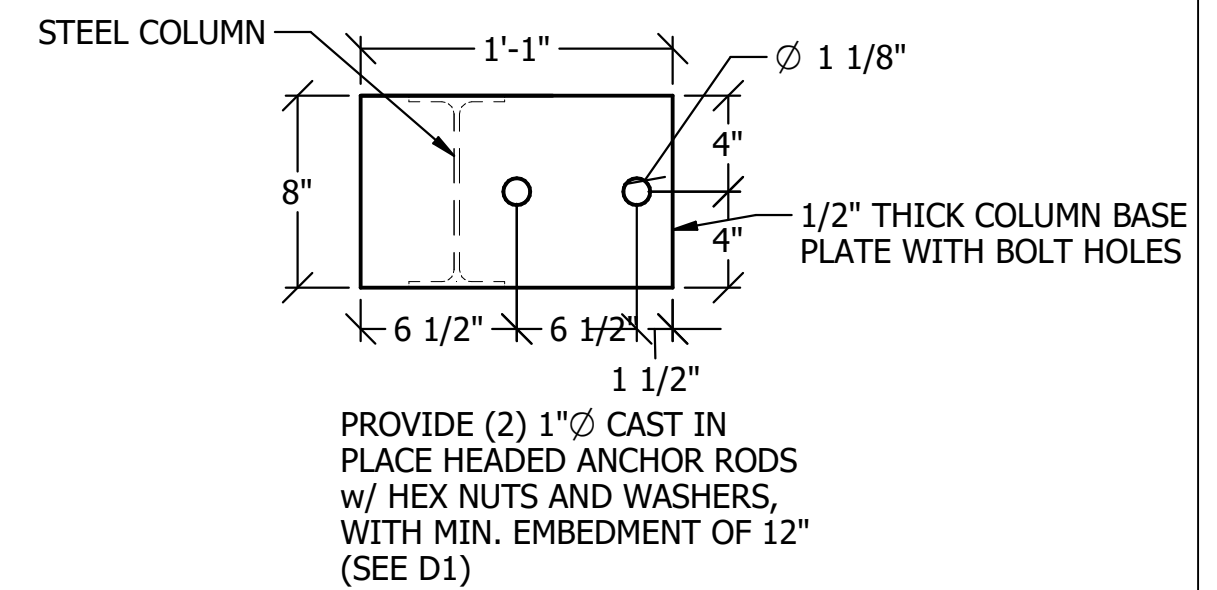
F1 FOUNDATION PLAN LAYOUT
SCALE: N.T.S.



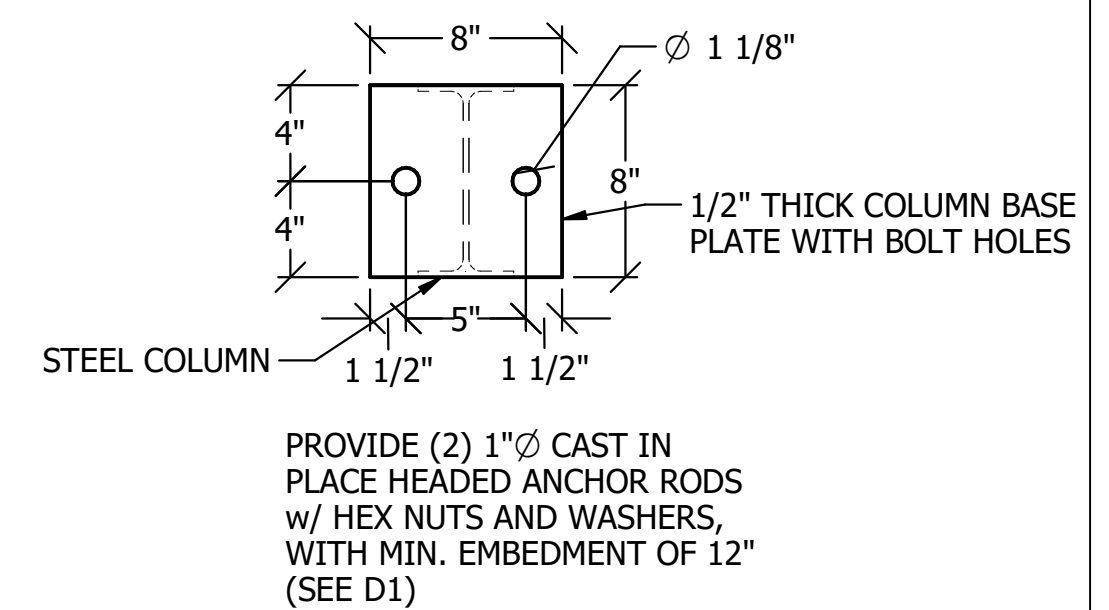
P4 COLUMN BASE PLATE D
SCALE: 1-1/2"=1'-0"



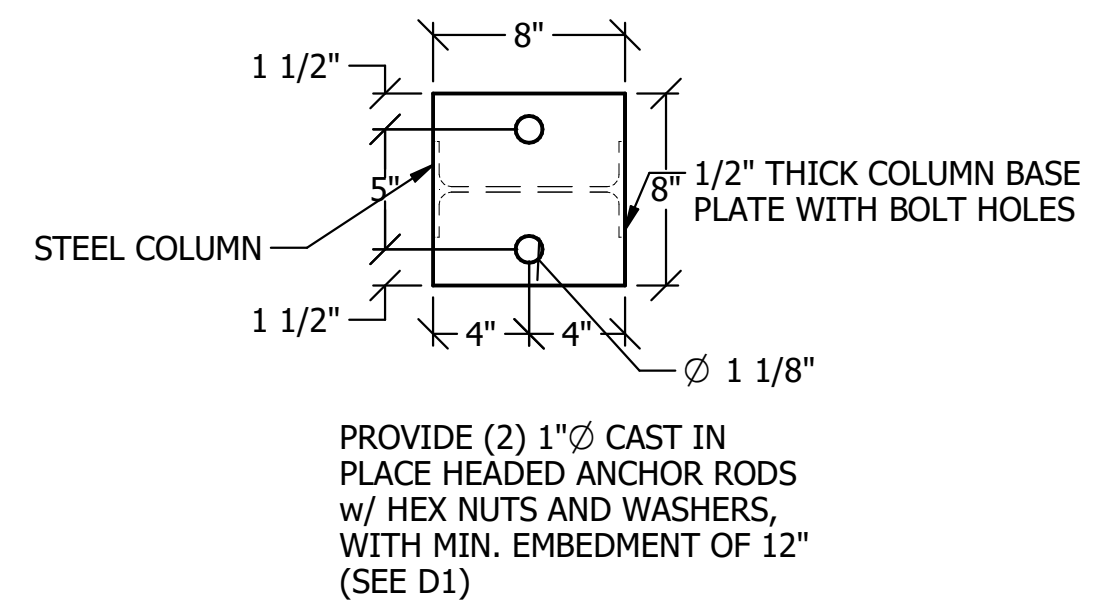
P5 COLUMN BASE PLATE E
SCALE: 1-1/2"=1'-0"



P1 COLUMN BASE PLATE A
SCALE: 1-1/2"=1'-0"



P2 COLUMN BASE PLATE B
SCALE: 1-1/2"=1'-0"



P3 COLUMN BASE PLATE C
SCALE: 1-1/2"=1'-0"

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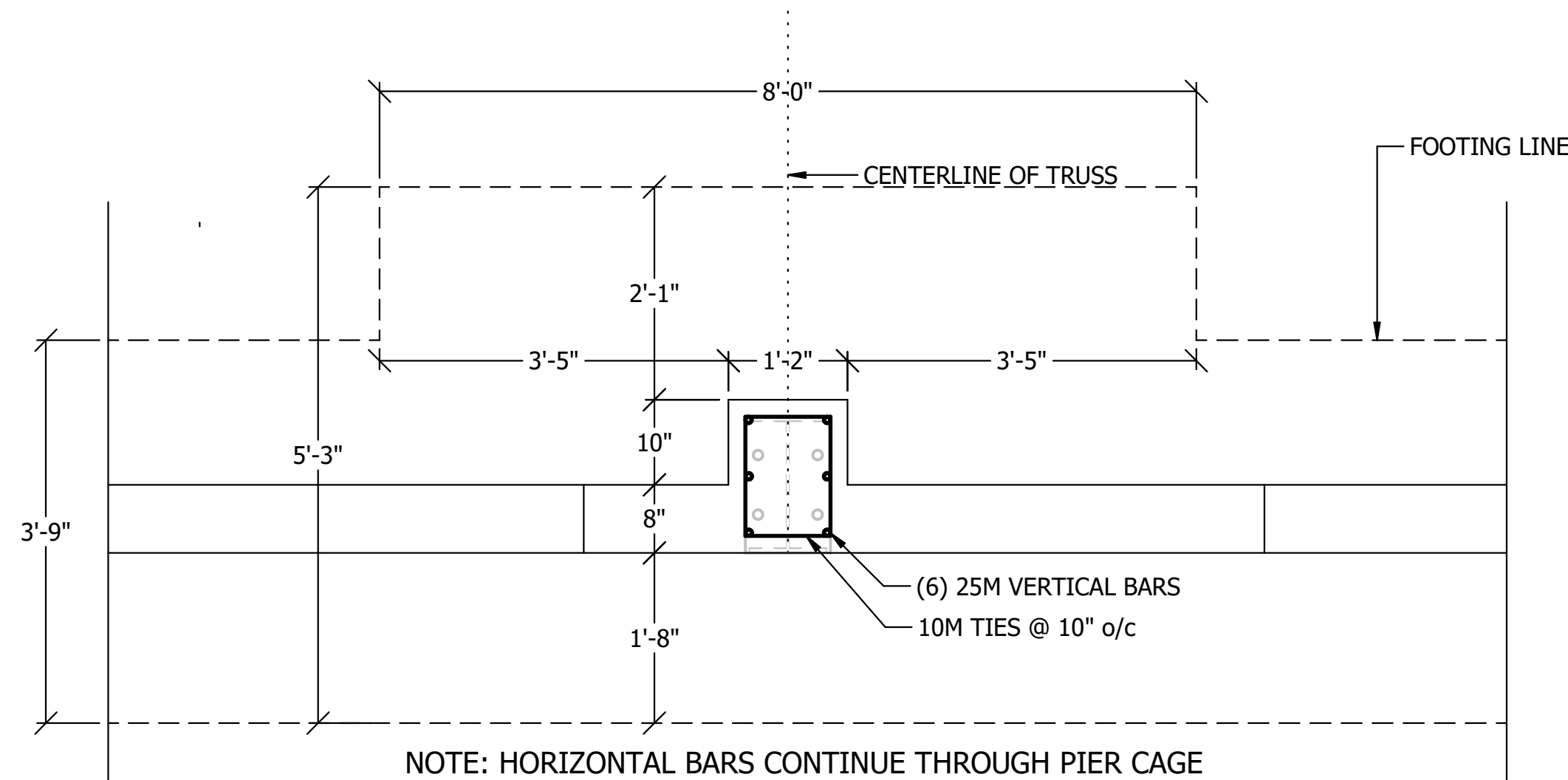
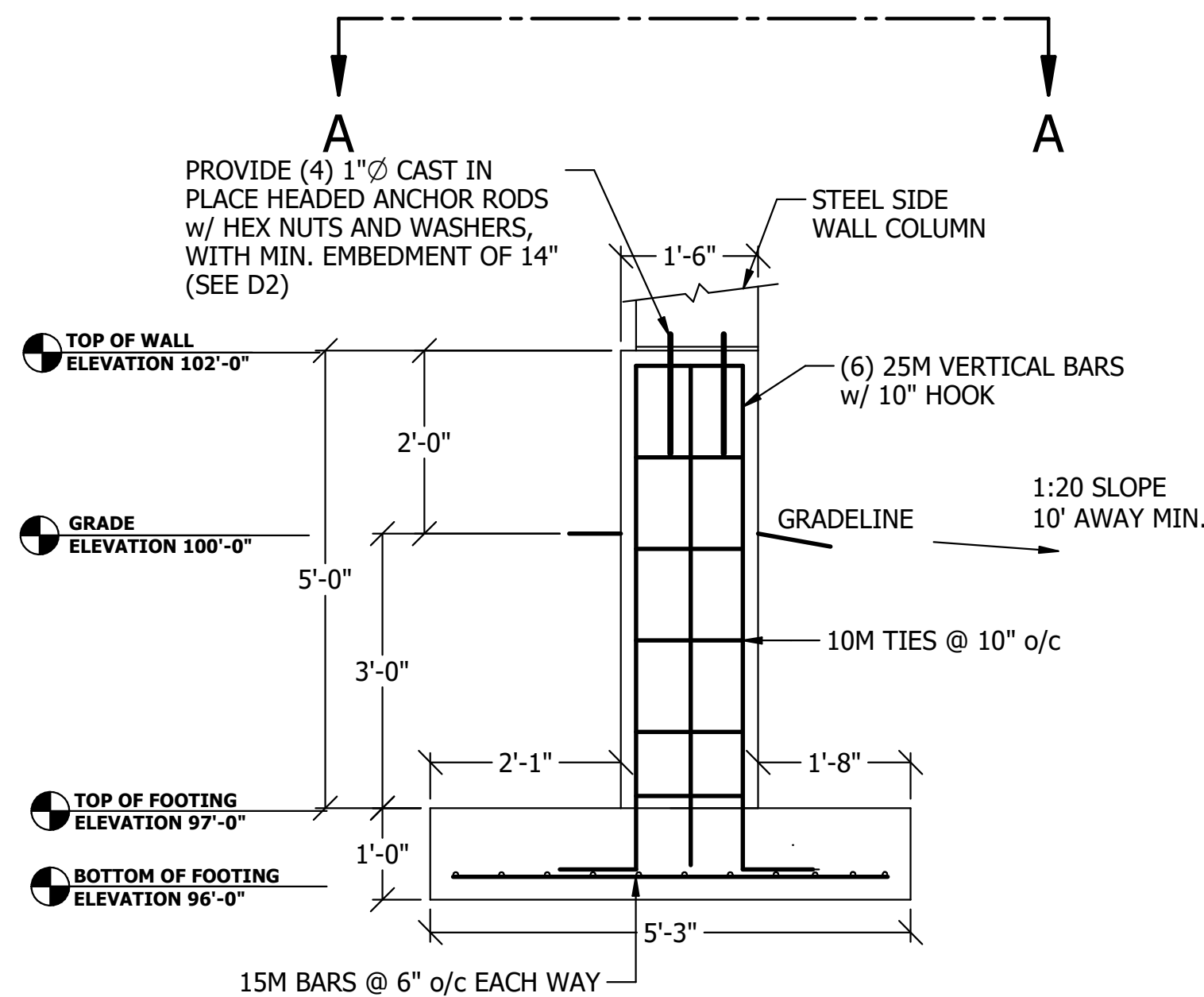
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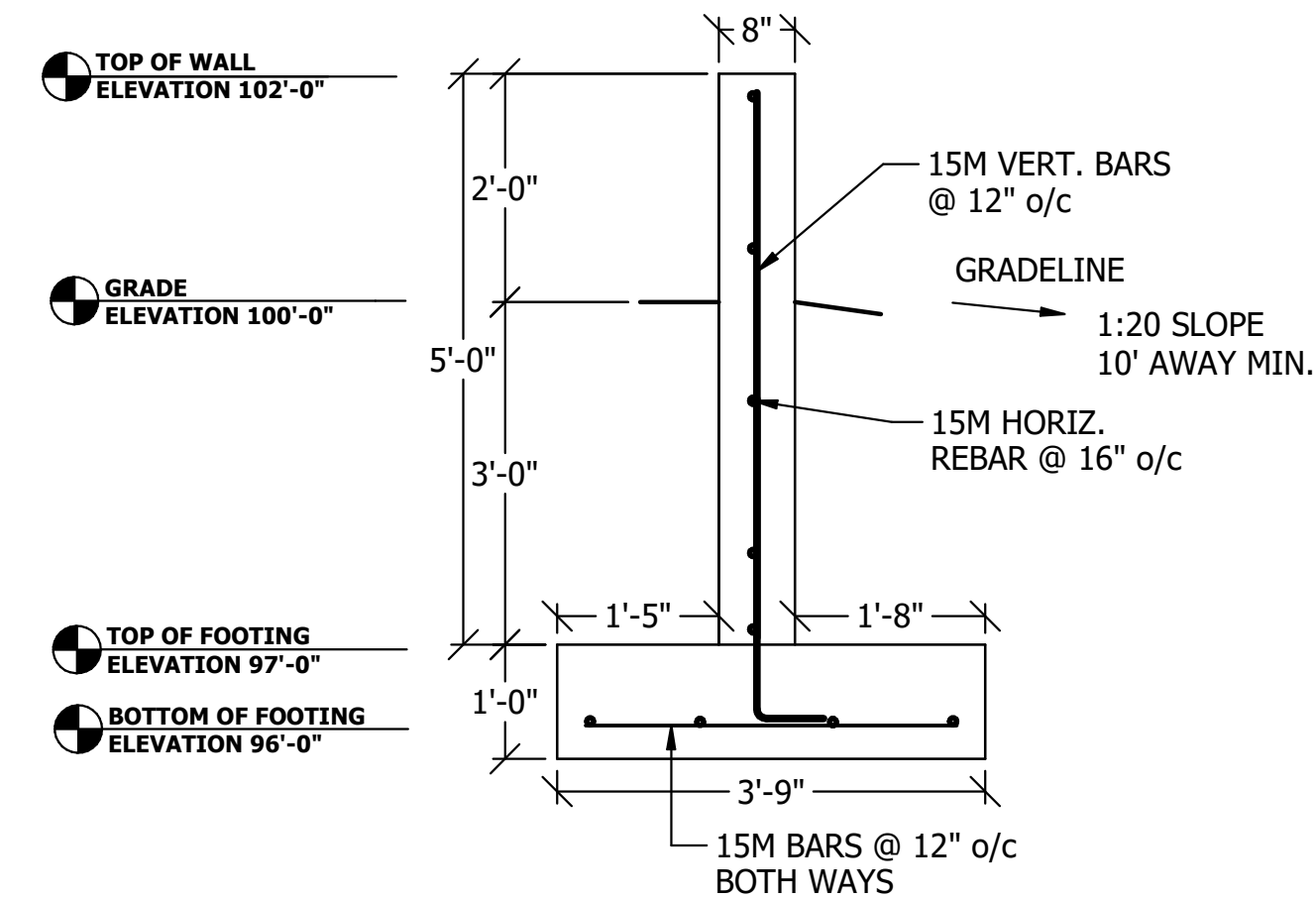
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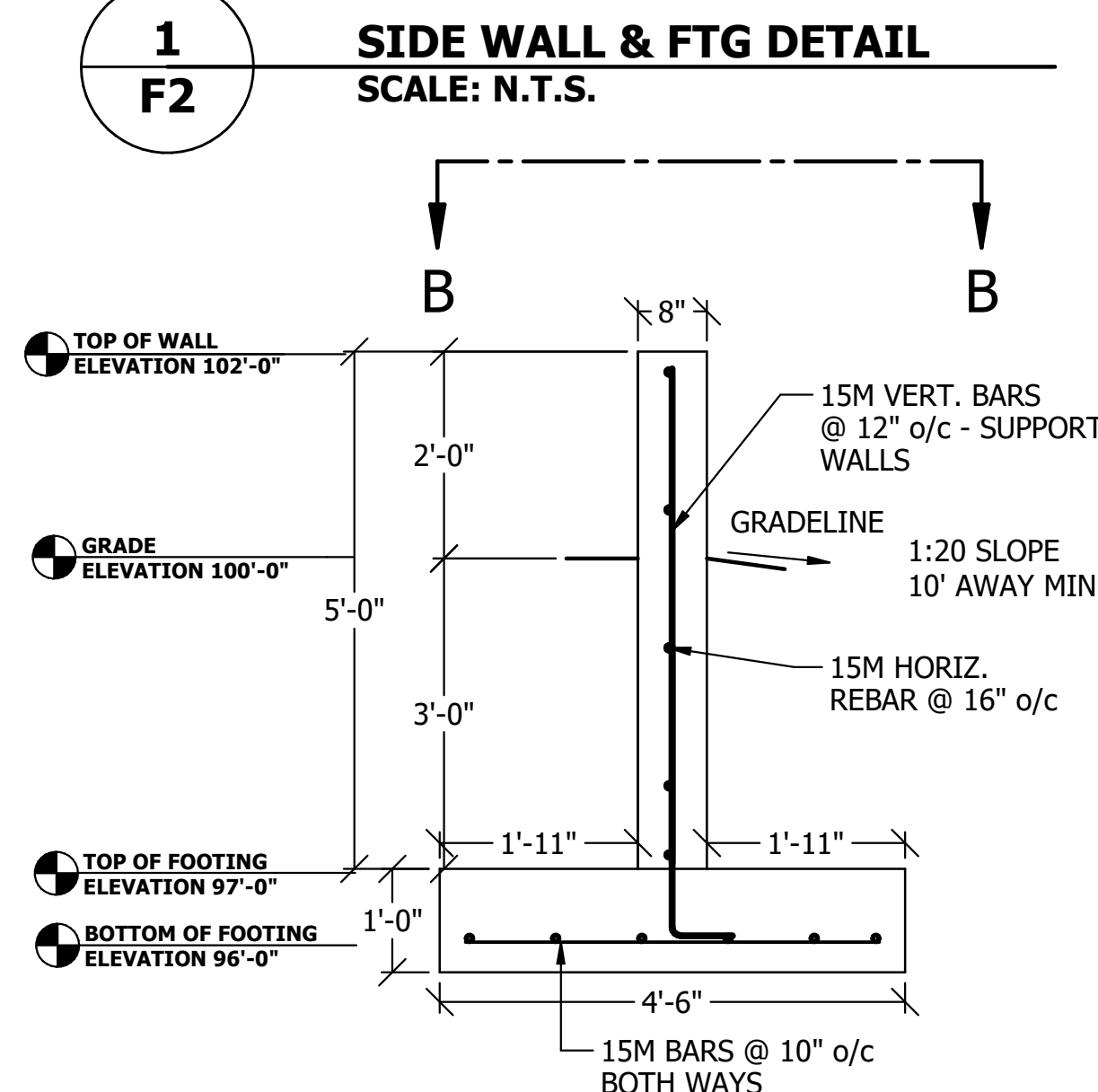
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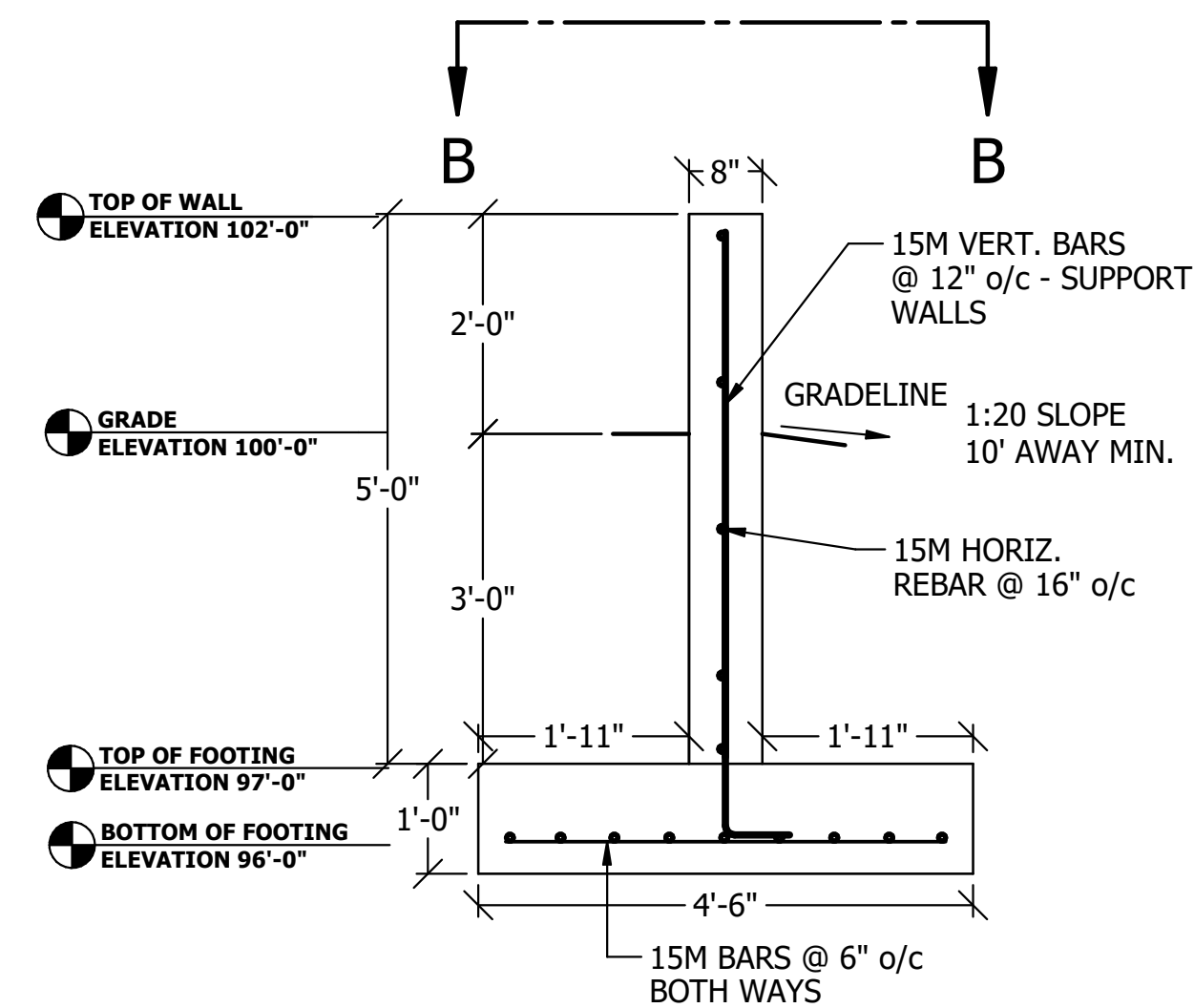
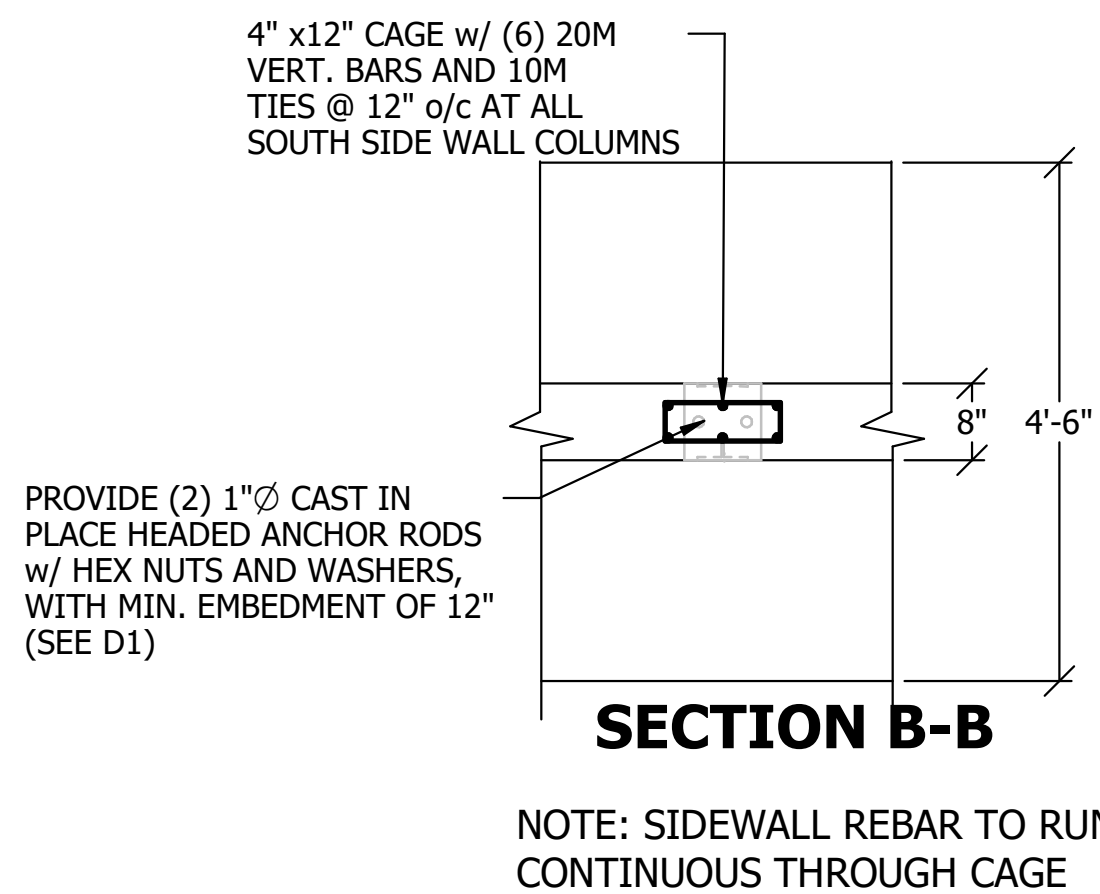
DETAIL A



2 SIDE WALL & FTG DETAIL (BETWEEN COLUMNS) SCALE: N.T.S.

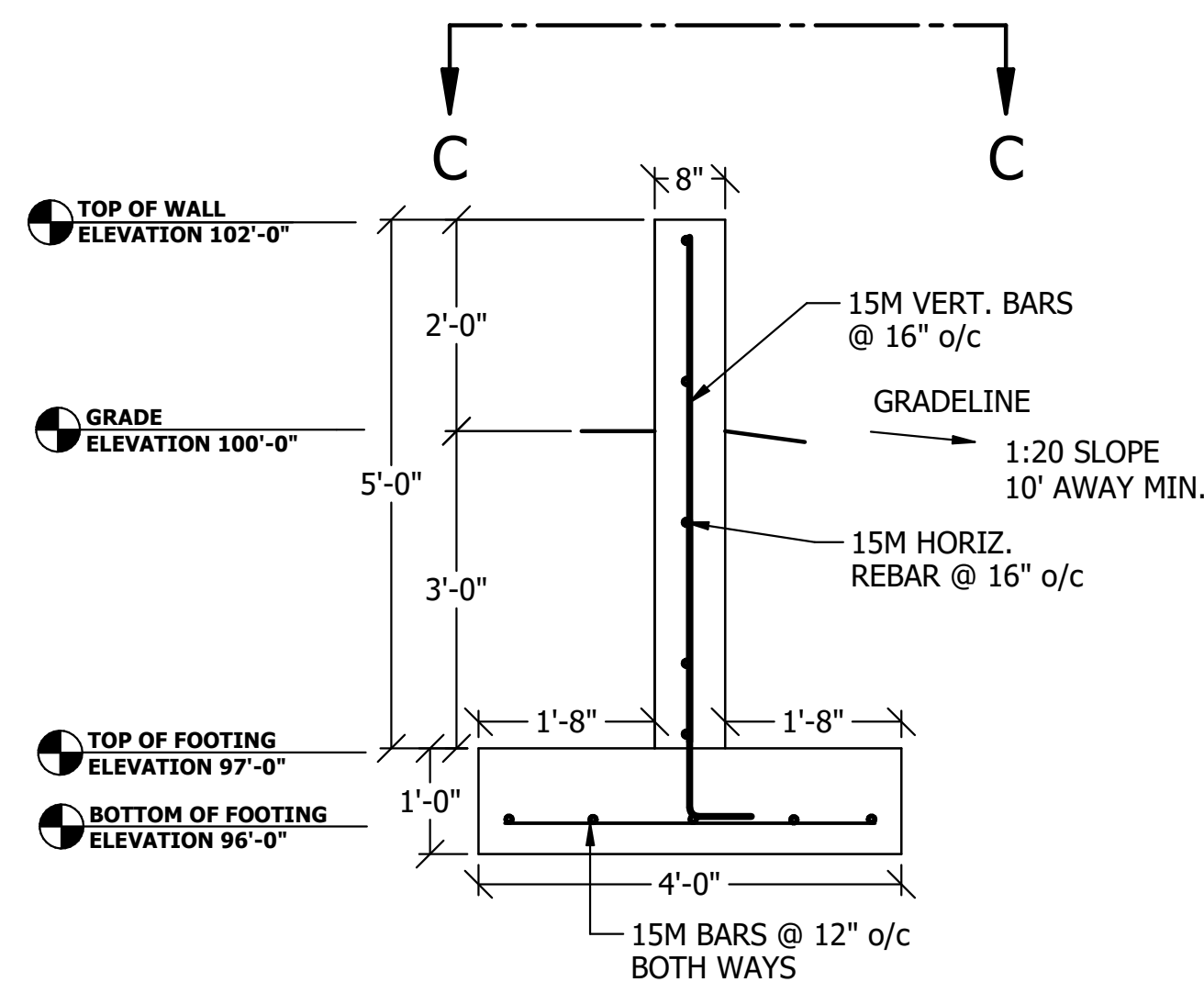


1 SIDE WALL & FTG DETAIL SCALE: N.T.S.

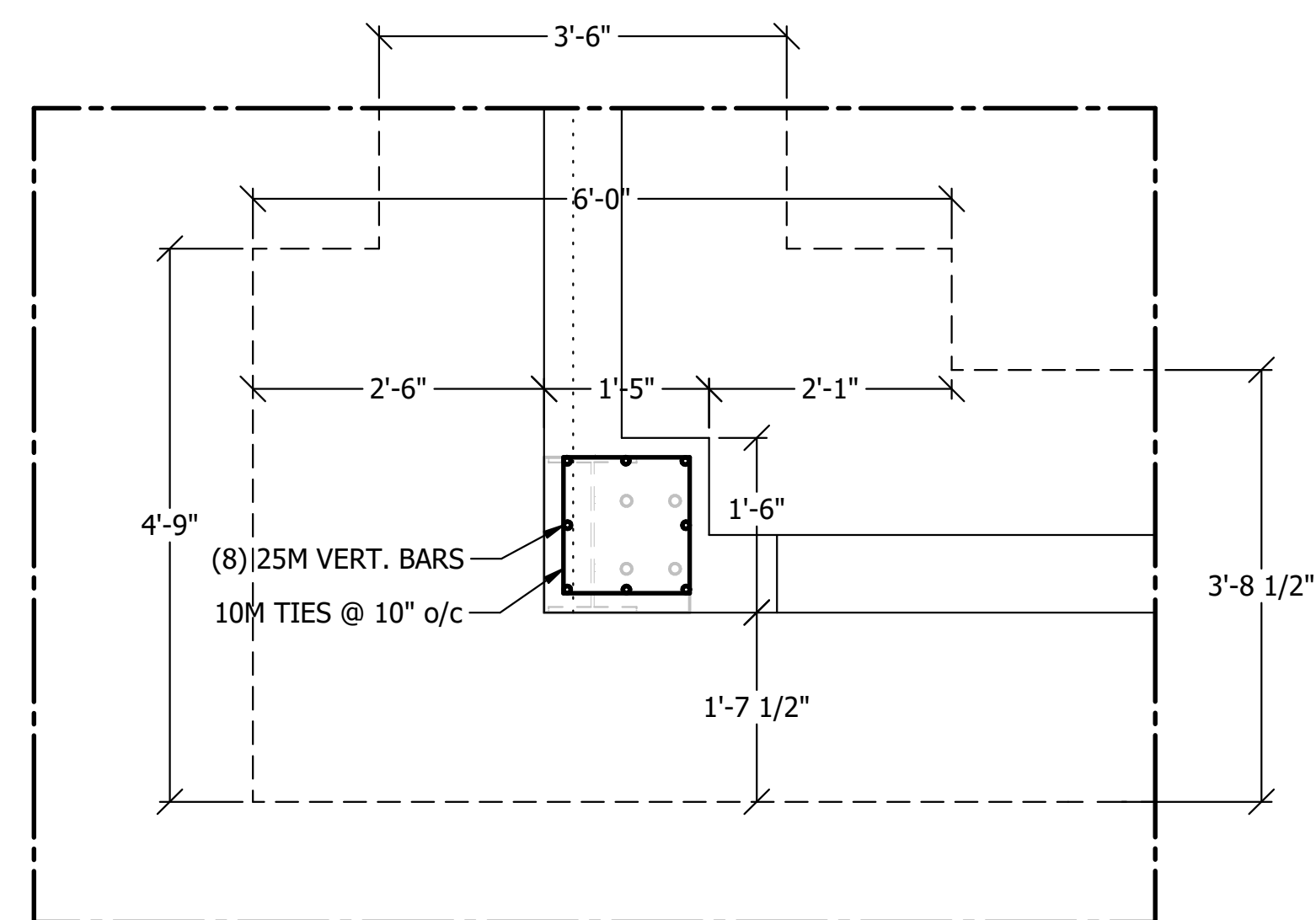
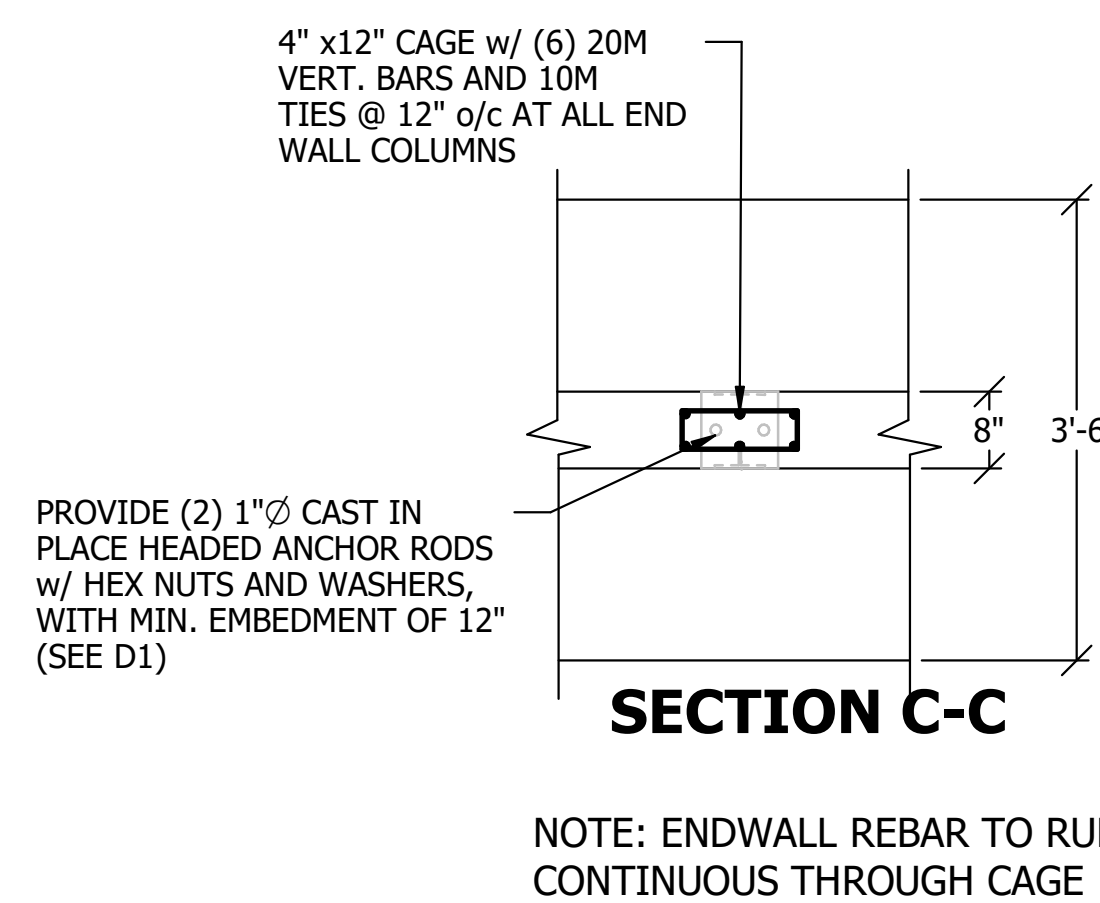


4 SIDE WALL & FTG DETAIL (COLUMNS 3 AND 7) SCALE: N.T.S.

3 SIDE WALL & FTG DETAIL (COLUMNS 1,2,4,5,6,8,9) SCALE: N.T.S.



5 END WALL & FTG DETAIL SCALE: N.T.S.



DETAIL D

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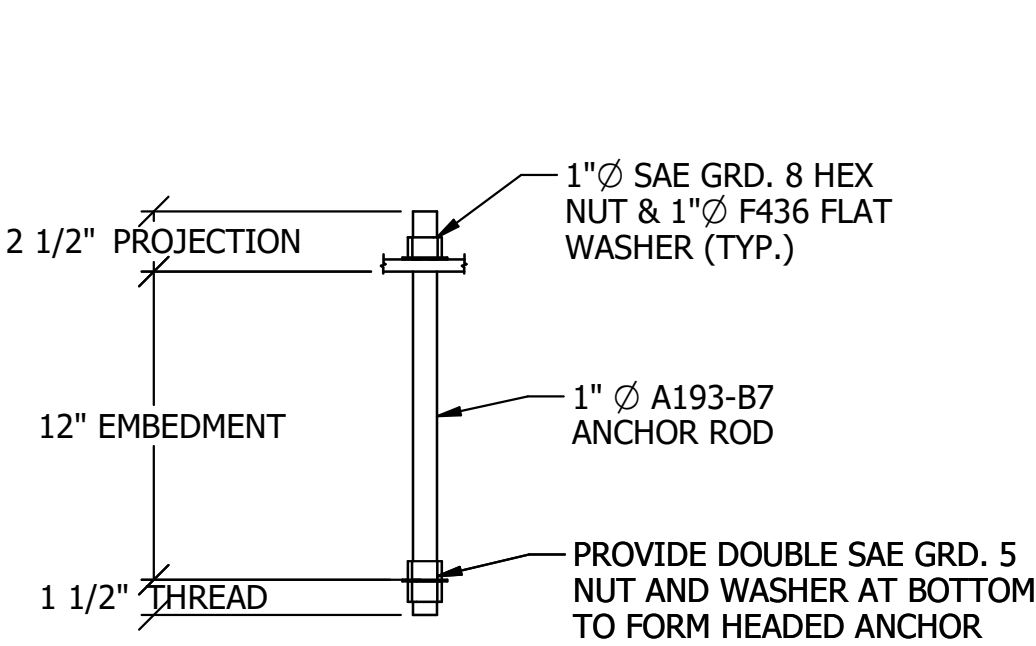
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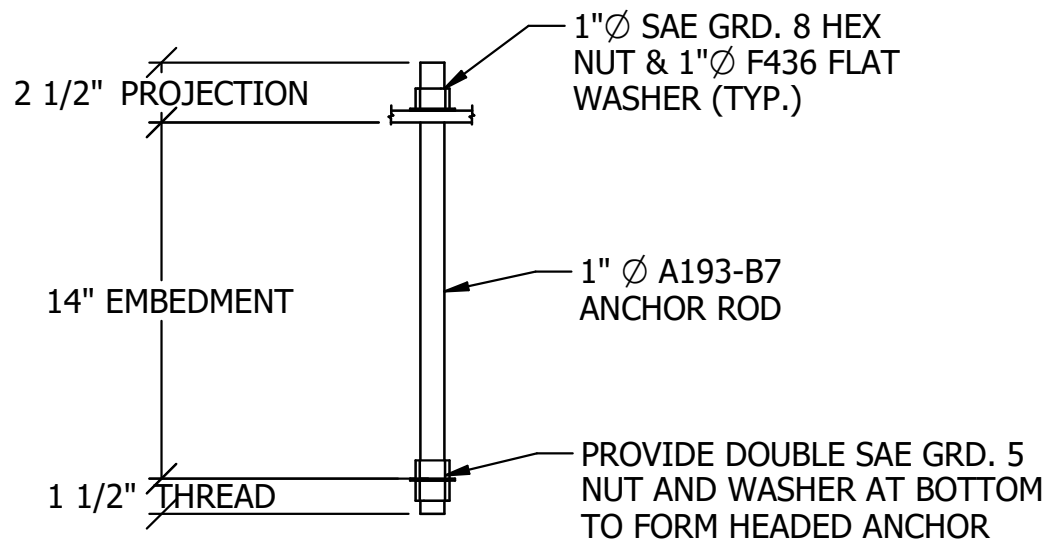
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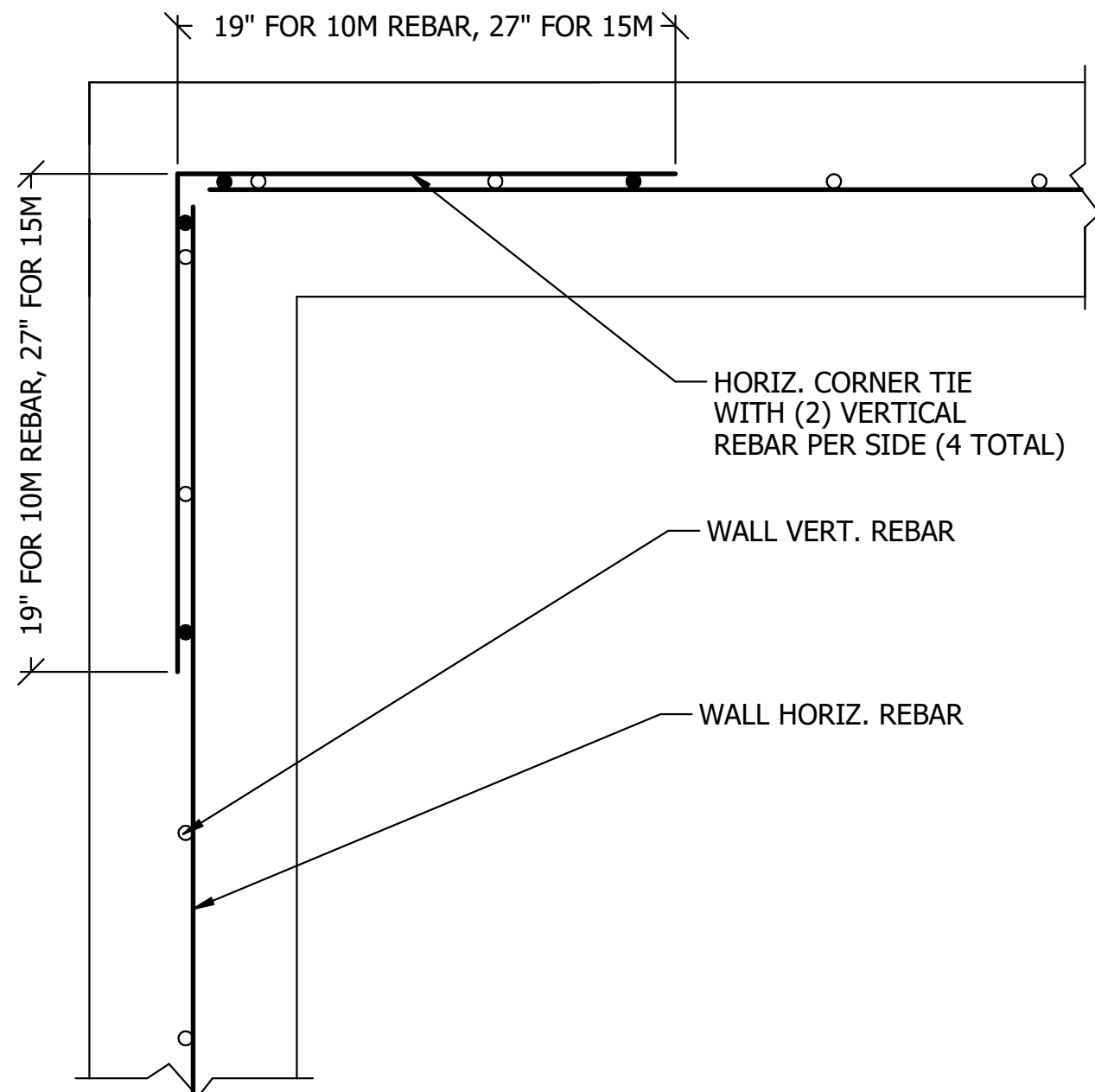
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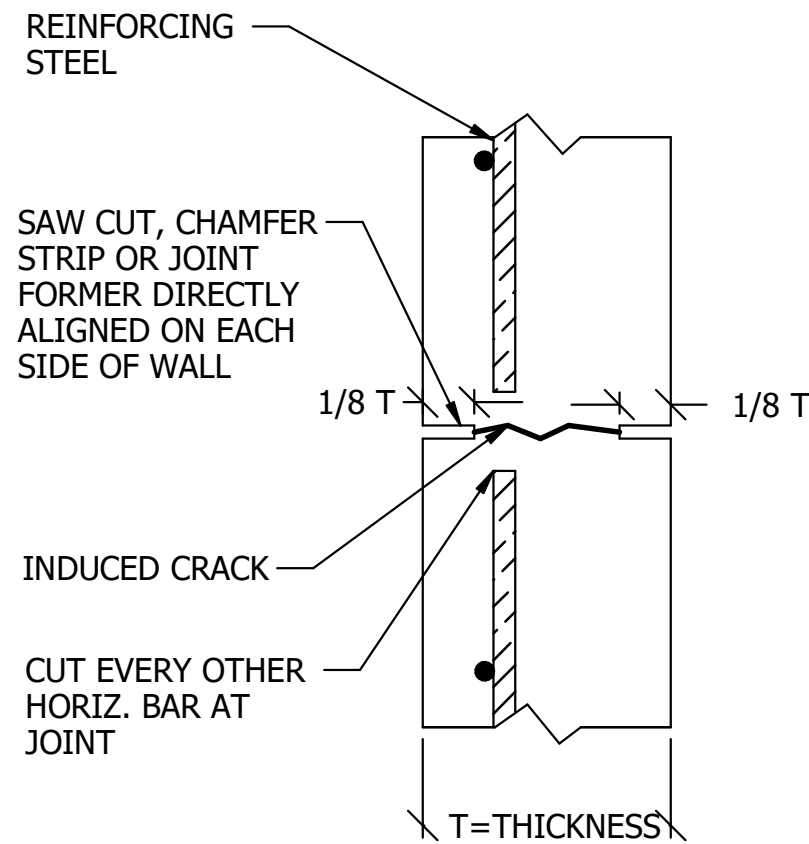
D1 ANCHOR BOLT DETAIL (TYP.)
SCALE: NTS



D2 ANCHOR BOLT DETAIL (TYP.)
SCALE: NTS

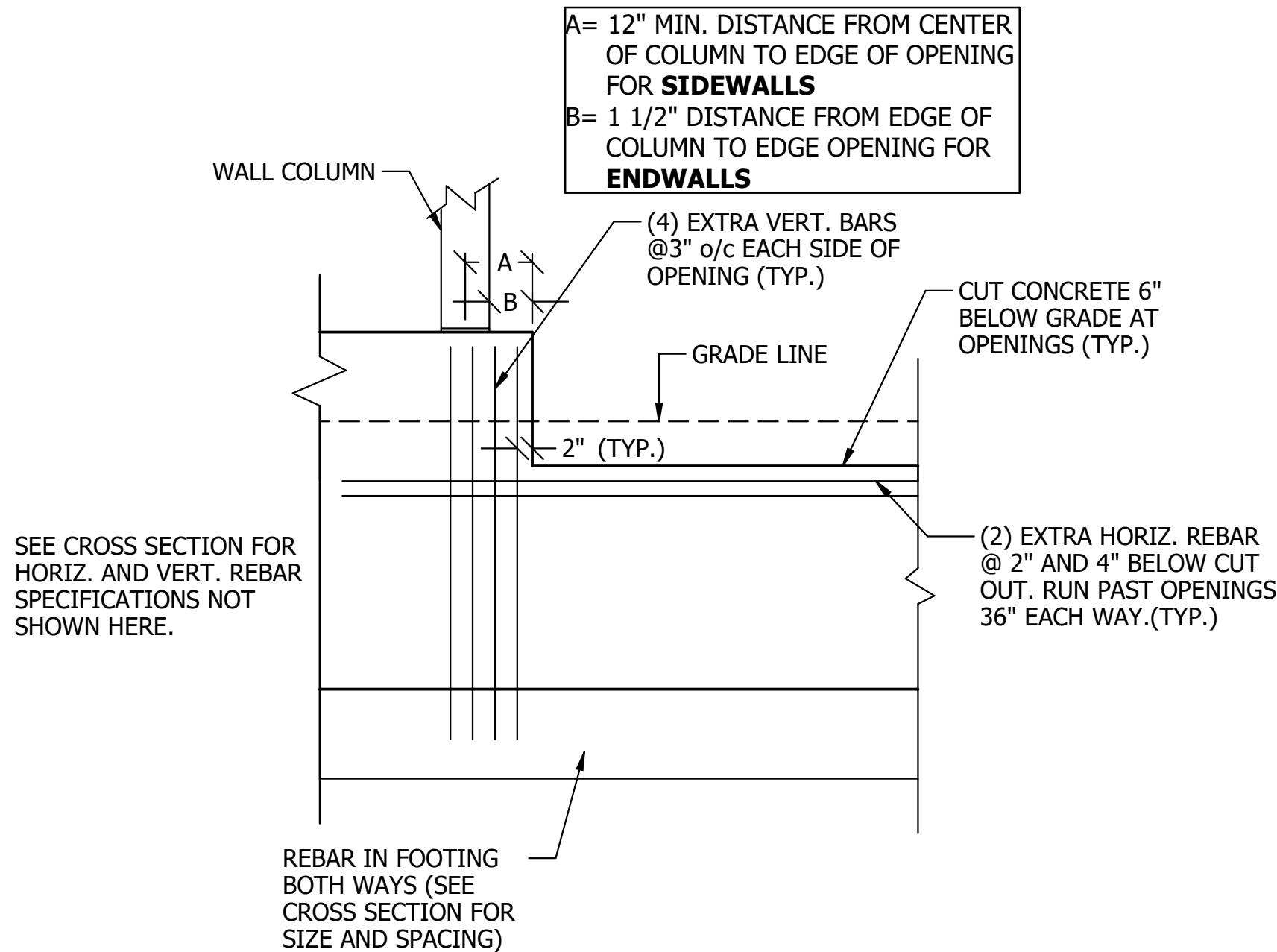


D3 SINGLE MAT CORNER DETAIL
SCALE: NTS



D4 WALL CONTROL JOINT
PLAN VIEW - SCALE: NTS

- NOTES:
1. T/8 NEED NOT BE GREATER THAN 1" FOR WALLS THICKER THAN 8"
 2. STRIP JOINT FORMERS, IF USED, AFTER AGGREGATE IS SET.
 3. SAWED JOINTS TO BE CUT SOON AFTER AGGREGATE IS SET (WITHIN 4-12 HOURS OF CONCRETE POUR)



D5 TYPICAL OPENING CUTOUT (U.N.O.)
SCALE: N.T.S.

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PROJECT START DATE:	LAST PRINT DATE:
6/23/2021	03/16/2022
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RIDGELINE 101.21.211S-ADD2	
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